

Foundation characteristics of the soils of different parts of Nepal

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ABSTRACT

This paper deals with foundation characteristics of the soils of different parts of Nepal. In this paper, multiple approaches were adopted to explore foundation characteristics of the soil. In this study 14 sites from different parts of the country were selected; 2 sites from the hilly region, 2 sites from the inner Terai and 10 sites from the Terai. In each site two test sites were selected. In each test site simplified penetration apparatus (SPA) tests were carried out and were accompanied by the auger tests. Soil samples from different depths in each site were collected for the direct shear test, soil classification, LL-PL test, density and other tests and these tests were carried out in laboratory. Bearing capacity of the soils thus obtained from the laboratory was compared with the soil types of certain depth and the Nc value at that depth. From the study it was found that the Nc value depends upon the types of the soil and the compactness of the soils. This study showed that Nc value can be converted in to the ultimate bearing capacity by multiplying the obtained Nc value by the factor of 35 within 80% confidence. Resistivity measurements were carried out only to explore the suitability of the sites for the purposed construction of substations in terms of earthing. Resistivity measurement showed that the sites are suitable for the construction of purposed substations.

Keywords: SPA, bearing capacity, subsurface soil, foundation, Nepal

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INTRODUCTION

This paper presents the results of the subsurface investigation works carried out for the proposed substation at different parts of Nepal (Fig. 1). Foundation characteristics of the soil depends upon the soil properties such as soil type, grading, liquid limit, plastic limit, density, cohesion, compactness of the soil in the layer, friction angle, etc. In this study a total of 14 sites were selected; 2 sites from the hilly region, 2 sites from the inner Terai and 10 sites from the Terai. In each site, two test sites were selected. In each test site simplified penetration apparatus (SPA) tests were carried out. This is the simple instrument designed for the shallow soil investigation. Here, this instrument was used to characterize the subsurface soil horizons using the Nc number (the no. of blows required to penetrate 10 cm. depth). For each of the site two tests were carried out so that direct comparison can be done. Each site is accompanied by the auger test so that direct observation of the soil at depth of penetration can be done. Cross litholog of each site along with the Nc value was used to prepare the detailed subsurface soil horizons. Soil samples from different depths in each site were collected for laboratory test. Undisturbed soil samples were collected for the direct shear test and disturbed samples were collected for

the soil classification, LL- PL test, density and other tests. Bearing capacity of the soil thus obtained from the laboratory was compared with the soil type of certain depth and the Nc value at that depth.

The proposed substation sites are as follows; 1. Bhiman Substation (Sindhuli), 2. Simroungadha Substation (Bara), 3. Katari Substation (Udayapur), 4. Yedukuwa Substation (Dhanusha), 5. Aurahai Substation (Mahotari), 6. Amuwa Substation (Kapilbastu), 7. Devdaha Substation (Rupandehi), 8. Mukundapur Substation (Nawalparasi), 9. Milan Chowk Substation (Parabat), 10. Jitpur Substation (Rupandehi), 11. Biratchowk Substation (Morang District), 12. Rangeli Substation (Morang District), 13. Phattepur (Balardaha) Substation (Saptari District), and 14. Phikal Substation (Ilam District).

For each substation, two sites - one for the control building and another for the Switchyard (Transformer Site) were selected. Wherever possible and necessary, both field test and laboratory test were conducted for each site. The objectives of this investigation are to study the subsurface character of the site and type, and loading intensity on the foundation for the proposed structure.

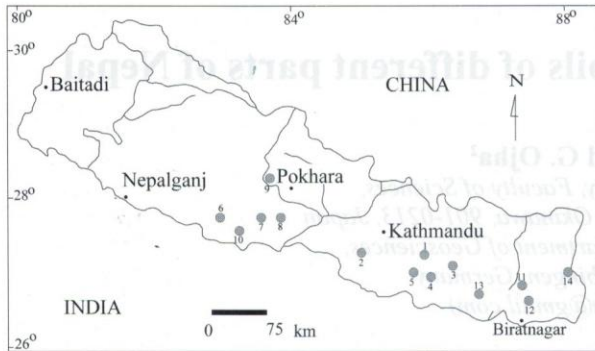


Fig. 1: Location map of the study sites

DETAILED METHODOLOGY

The field work is comprised of the following components; detail survey, preparation of the site map, simplified penetration test up to the depth of 6 m if possible, preparation

of litholog of Hand Auger holes up to the depth of 6 m if possible, location of sites on the prepared maps, collection of samples for the laboratory test, measurement of resistivity of the site and water table measurement. Similarly, the laboratory tests comprise the following tests; moisture content, sieve analysis, Liquid Limit (LL) - Plastic Limit (PL), classification of the soil (UCS Classification), specific gravity, Direct Shear Test, and calculation of bearing capacity.

After careful study of both the data collected during the field investigation and the results obtained from the laboratory tests, strength and other soil properties were determined for the design of the foundation. Furthermore, the paper gives the soil properties of the each layer of subsurface deposits.

The permissible loading intensity for most suitable type of foundation has been evaluated. From the field data and the laboratory results, the safe bearing capacity in general for the strip 1.2 m wide, isolated 2 m x 2 m, Raft 5 m x 5 m for all the sites are calculated and are documented in the chapter "bearing capacity of the soil".

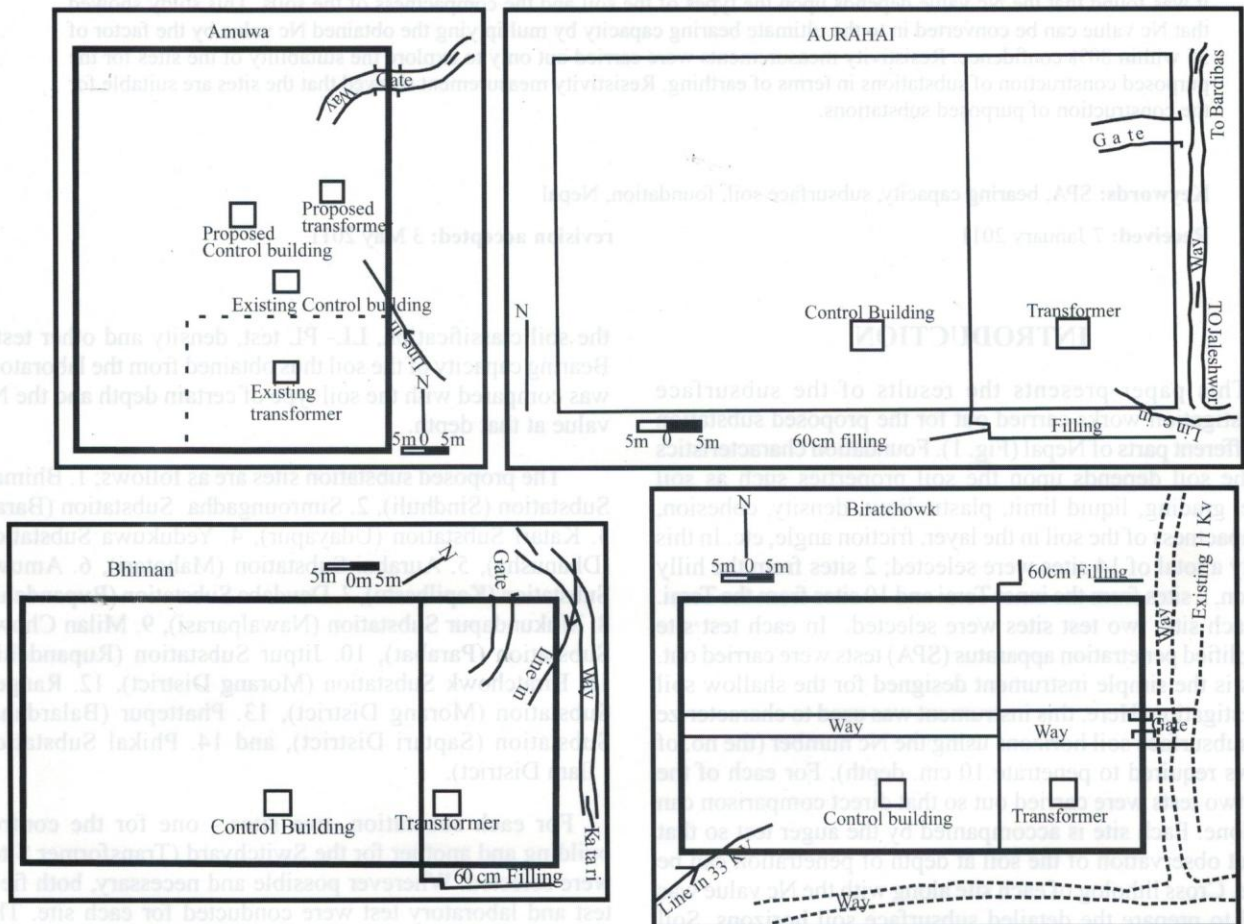


Fig. 2: Plan maps of sites : (a) Amuwa, Aurahai, Bhiman and Biratchowk

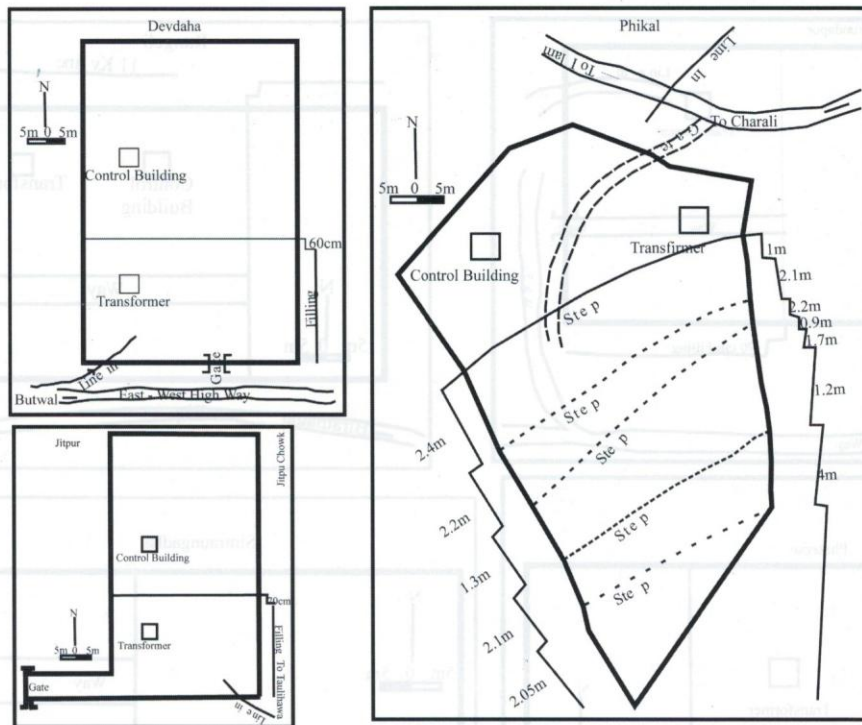


Fig. 2: Plan maps of sites : (b) Devdaha, Jitpur, and Phikal

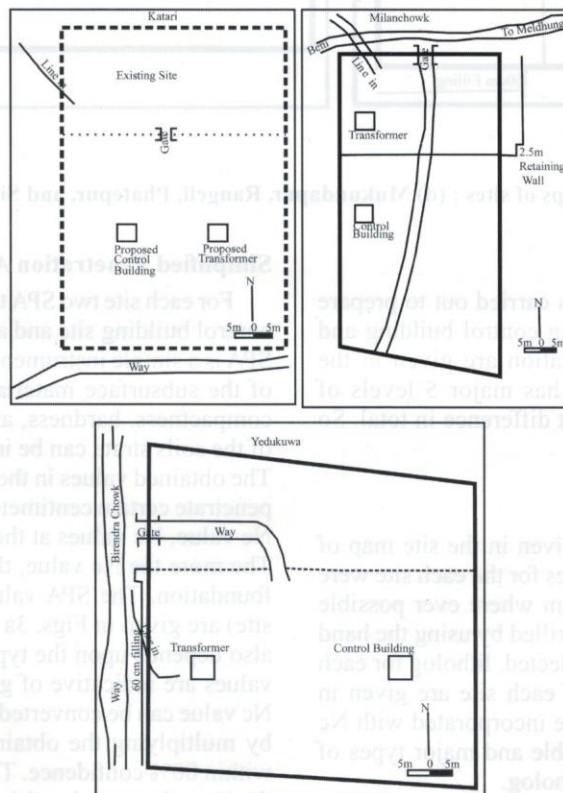


Fig. 2: Plan maps of sites : (c) Katari, Milanchowk and Yedukuwa

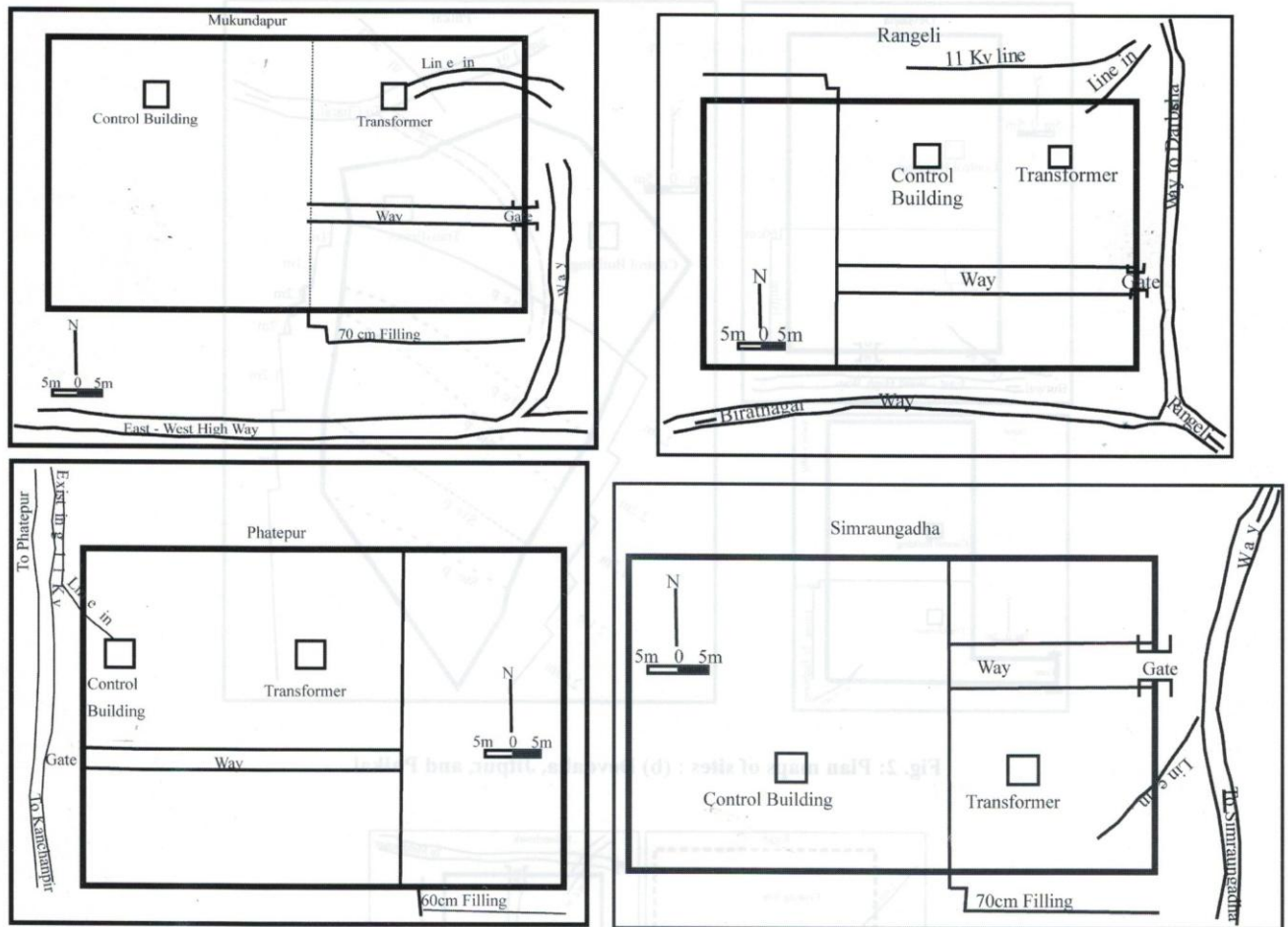


Fig. 2: Plan maps of sites : (d) Mukundapur, Rangeli, Phatepur, and Simraungadha

Detail survey

Detail survey of all the sites was carried out to prepare the site map. The site maps showing control building and switchyard position for each substation are given in the Fig. 2. The Phikal substation site has major 5 levels of terraces with more than 10 m height difference in total. So it should be leveled.

Boreholes

Location of the bore holes are given in the site map of each substation. A total of 2 bore holes for the each site were drilled at the maximum depth of 6 m where ever possible and necessary. Each bore hole was drilled by using the hand auger and using soil sample thus collected, litholog for each site were prepared. The litholog of each site are given in the Figs. 3a to 3z. These litholog are incorporated with Nc value verses depth curves. Water table and major types of soil horizons are described in the litholog.

Simplified Penetration Apparatus (SPA) test

For each site two SPA tests were carried out; one test at the control building site and another test at the switchyard area. SPA is a simple instrument for the continuous measurements of the subsurface material. The differences in the density, compactness, hardness, and other important characteristics of the soils strata can be interpreted by using the SPA value. The obtained values in the field, i. e., the number of blows to penetrate certain centimeter at certain depth are converted as Nc value. Nc values at the given depth are then interpreted. The more the Nc value, the layer of the soil is good for the foundation. The SPA values of each bore hole (2 for each site) are given in Figs. 3a to 3z. The Nc value thus obtained also depends upon the type of the soil but generally, higher values are indicative of good layer for the foundation. The Nc value can be converted in to the ultimate bearing capacity by multiplying the obtained Nc value by the factor of 35 within 80% confidence. The correlation coefficient between the Nc value and the ultimate bearing capacity is 0.829522. Therefore, the safe bearing capacity can be obtained from

the converted Nc value by dividing them by the factor of safety (3). The SPA tests should be repeated to as to obtain unbiased values. Results of the bearing capacity from the Nc value are given in Table 1.

Soil sampling

Two types of the sampling were done; disturbed sampling and undisturbed sampling. The laboratory test includes the following; sieve analysis, LL – PL test, natural moisture content, specific gravity, and direct shear test. Disturbed samples were collected for laboratory tests such as sieve analysis, LL-PL test, moisture content, and specific gravity. Undisturbed samples were collected for direct shear test. The undisturbed samples were collected in the tube by hammering the tube on the material in the boreholes. The sample lifted from the depth is sealed with the wax at the both ends of the tube to prevent the moisture loss and taken to the laboratory for the testing.

In-situ testing

The in-situ testing was performed by the SPA test. For each site, it was continuously done down to the depth of 6 m if it is possible and necessary. The SPA data shows that the soil at the Phikal site is very loose up to the depth of 1 m. Thus the first 1 m soil should be removed totally and due consideration should be given during detail design stage. The SPA test data can be used to compare the bearing capacity of the soil further down to the depth of 6 m.

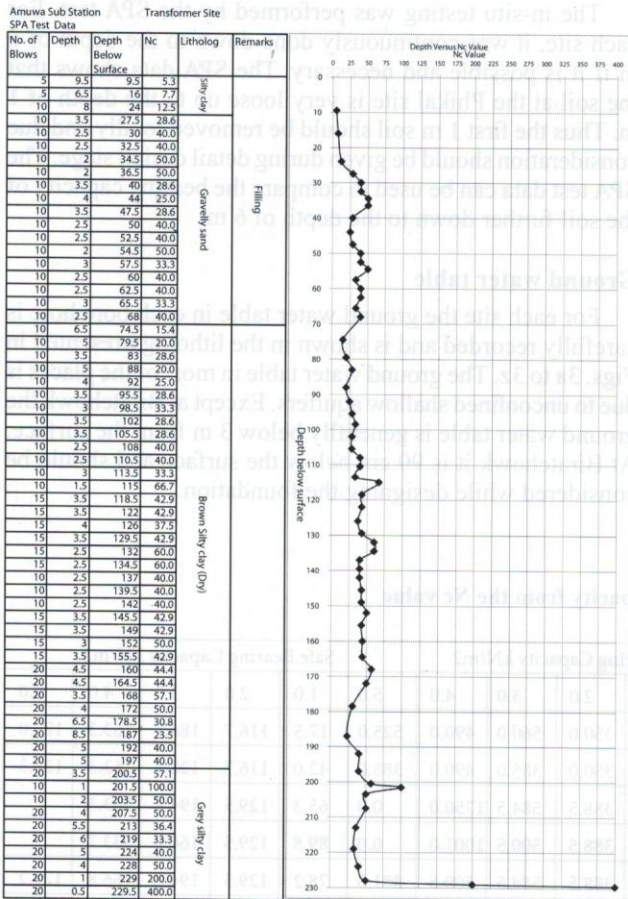
Ground water table

For each site the ground water table in each bore hole is carefully recorded and is shown in the litholog presented in Figs. 3a to 3z. The ground water table in most of the places is due to unconfined shallow aquifers. Except at Biratchowk the ground water table is generally below 3 m² from the surface. At Biratchowk it is 90 cm below the surface and should be considered while designing the foundation.

Table 1: Results of bearing capacity from the Nc value

| Location / Depth (m) | Minimum average Nc | | | | | Bearing Capacity kN/m2 | | | | | Safe Bearing Capacity kN/m2 | | | | |
|-------------------------------|--------------------|------|------|------|------|------------------------|--------|-------|--------|--------|-----------------------------|-------|-------|-------|-------|
| | 1.0 | 2.0 | 3.0 | 4.0 | 5.0 | 1.0 | 2.0 | 3.0 | 4.0 | 5.0 | 1.0 | 2.0 | 3.0 | 4.0 | 5.0 |
| Phikal Transformer Site | 1.5 | 10.0 | 16.0 | 14.0 | 15.0 | 52.5 | 350.0 | 560.0 | 490.0 | 525.0 | 17.5 | 116.7 | 186.7 | 163.3 | 175.0 |
| Phikal Control Building Site | 3.6 | 10.0 | 11.0 | 14.0 | 11.0 | 126.0 | 350.0 | 385.0 | 490.0 | 385.0 | 42.0 | 116.7 | 128.3 | 163.3 | 128.3 |
| Aurahi Control Building | 5.6 | 11.1 | 16.7 | 50.0 | | 196.0 | 388.5 | 584.5 | 1750.0 | 0.0 | 65.3 | 129.5 | 194.8 | 583.3 | |
| Aurahi Transformer | 7.7 | 11.1 | 14.3 | 28.6 | | 269.5 | 388.5 | 500.5 | 1001.0 | 0.0 | 89.8 | 129.5 | 166.8 | 333.7 | |
| Devdaha Control Building | 6.7 | 11.1 | 16.7 | 14.3 | 10.9 | 234.5 | 388.5 | 584.5 | 500.5 | 381.5 | 78.2 | 129.5 | 194.8 | 166.8 | 127.2 |
| Devdaha Transformer | 6.7 | 11.5 | 23.1 | 14.3 | 30.0 | 234.5 | 402.5 | 808.5 | 500.5 | 1050.0 | 78.2 | 134.2 | 269.5 | 166.8 | 350.0 |
| Simraungadha Transformer | 5.3 | 12.5 | 16.7 | 22.0 | | 185.5 | 437.5 | 584.5 | 770.0 | 0.0 | 61.8 | 145.8 | 194.8 | 256.7 | |
| Simraungadha Control Building | 8.3 | 10.5 | 12.5 | 16.7 | 20.0 | 290.5 | 367.5 | 437.5 | 584.5 | 700.0 | 96.8 | 122.5 | 145.8 | 194.8 | 233.3 |
| Jitpur Transformer | 6.3 | 17.9 | | | | 220.5 | 626.5 | 0.0 | 0.0 | 0.0 | 73.5 | 208.8 | | | |
| Jitpur Control Building | 14.0 | 16.0 | 22.2 | | | 490.0 | 560.0 | 777.0 | 0.0 | 0.0 | 163.3 | 186.7 | 259.0 | | |
| Mukundapur Transformer | 5.0 | 10.5 | 14.3 | 16.7 | 27.0 | 175.0 | 367.5 | 500.5 | 584.5 | 945.0 | 58.3 | 122.5 | 166.8 | 194.8 | 315.0 |
| Mukundapur control Building | 5.6 | 13.3 | 10.5 | 22.2 | 28.6 | 196.0 | 465.5 | 367.5 | 777.0 | 1001.0 | 65.3 | 155.2 | 122.5 | 259.0 | 333.7 |
| Amuwa Transformer Site | 5.3 | 23.5 | | | | 185.5 | 822.5 | 0.0 | 0.0 | 0.0 | 61.8 | 274.2 | | | |
| Rangeli Transformer | 8.3 | 10.0 | 16.7 | 22.2 | 16.7 | 290.5 | 350.0 | 584.5 | 777.0 | 583.5 | 96.8 | 116.7 | 194.8 | 259.0 | 194.5 |
| Rangeli control Building | 6.3 | 10.2 | 16.0 | 20.0 | 25.0 | 220.5 | 357.0 | 560.0 | 700.0 | 875.0 | 73.5 | 119.0 | 186.7 | 233.3 | 291.7 |
| Phatepur Transformer | 5.7 | 11.1 | 16.7 | 20.0 | 25.0 | 199.5 | 388.5 | 584.5 | 700.0 | 875.0 | 66.5 | 129.5 | 194.8 | 233.3 | 291.7 |
| Phatepur Control building | 8.3 | 12.5 | 16.7 | 28.6 | 33.3 | 290.5 | 437.5 | 584.5 | 1001.0 | 1166.6 | 96.8 | 145.8 | 194.8 | 333.7 | 388.9 |
| Biratchowk Transformer | 6.3 | 11.1 | 20.0 | 40.0 | 50.0 | 220.5 | 388.5 | 700.0 | 1400.0 | 1750.0 | 73.5 | 129.5 | 233.3 | 466.7 | 583.3 |
| Biratchowk Control Building | 3.7 | 11.0 | 25.0 | 40.0 | | 129.5 | 385.0 | 875.0 | 1400.0 | | 43.2 | 128.3 | 291.7 | 466.7 | |
| Yedukuwa Control Building | 10.0 | 11.1 | 16.7 | 28.6 | | 350.0 | 388.5 | 584.5 | 1000.0 | 0.0 | 116.7 | 129.5 | 194.8 | 333.3 | |
| Yedukuwa Transformer | 9.1 | 11.1 | 16.7 | 25.0 | | 318.5 | 388.5 | 584.5 | 875.0 | 0.0 | 106.2 | 129.5 | 194.8 | 291.7 | |
| Bhiman Control Building | 7.1 | 25.0 | | | | 248.5 | 875.0 | 0.0 | 0.0 | 0.0 | 82.8 | 291.7 | 0.0 | 0.0 | 0.0 |
| Bhiman Transformer | 2.9 | 20.0 | | | | 101.5 | 700.0 | 0.0 | 0.0 | 0.0 | 33.8 | 233.3 | | | |
| Milanchowk Control Building | 8.3 | 16.7 | 25.0 | | | 290.5 | 584.5 | 875.0 | 0.0 | 0.0 | 96.8 | 194.8 | 291.7 | | |
| Milanchowk Transformer | 8.3 | 18.2 | | | | 290.5 | 637.0 | 0.0 | 0.0 | 0.0 | 96.8 | 212.3 | | | |
| Katari | 15.4 | 33.3 | | | | 539.0 | 1165.5 | 0.0 | 0.0 | 0.0 | 179.7 | 388.5 | | | |

(a)



(b)

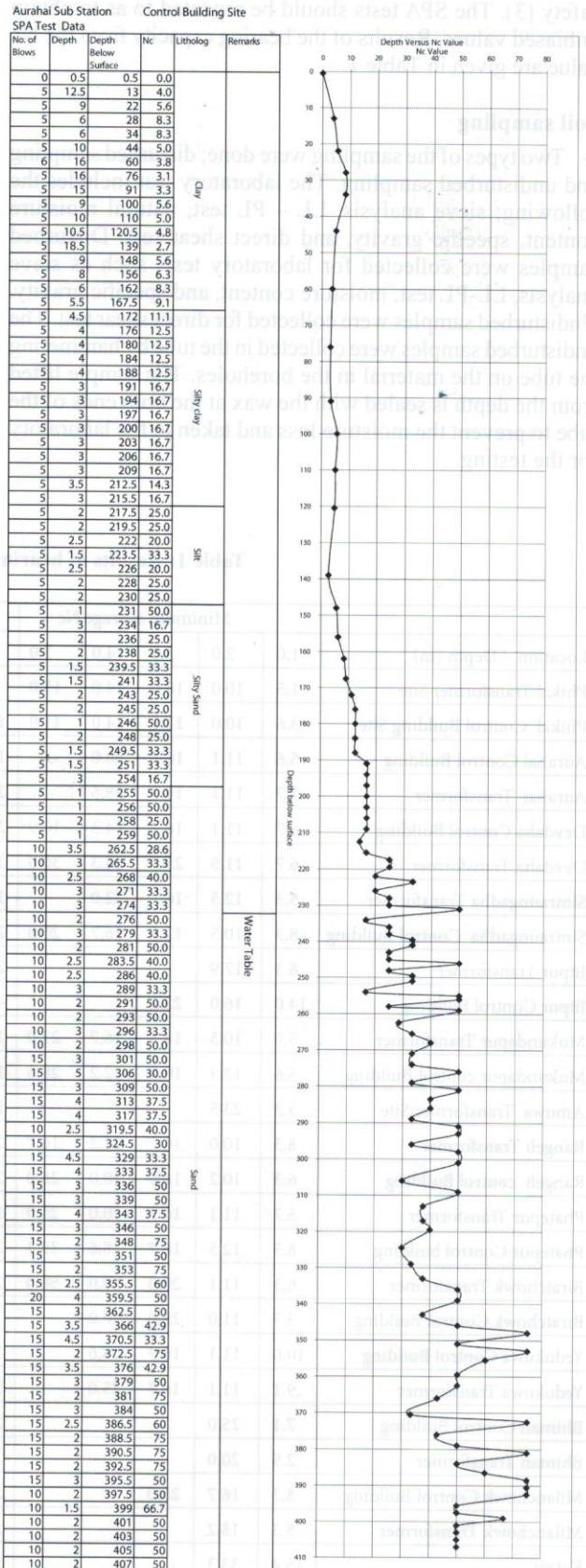


Fig. 3: Litholog, SPA test data, Nc value, Nc value Vs depth curve:

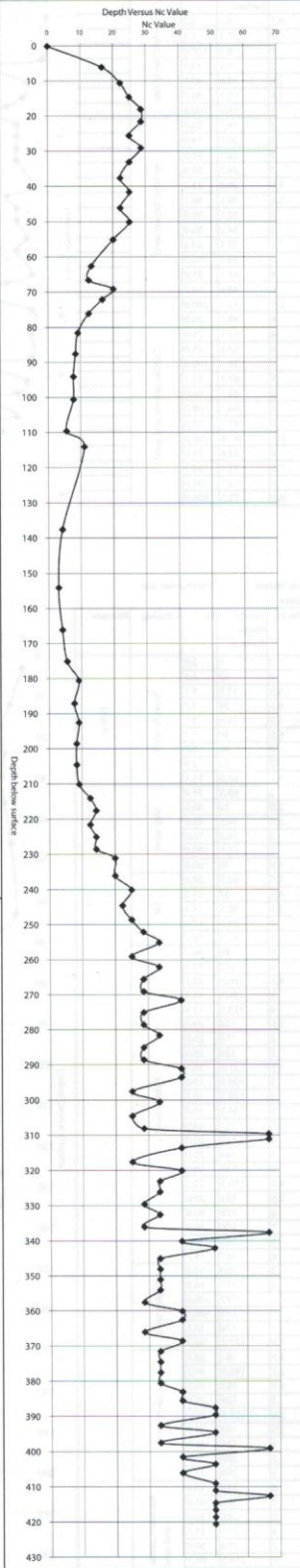
- (a) Amuwa Substation Transformer Site, (b) Auraha Substation, (c) Auraha Substation Transformer Site, (d) Phattepur (Balardaha) Substation Control Building Site, (e) Phattepur (Balardaha) Substation Transformer Site, (f) Bhiman Substation Control Building Site, (g) Bhiman Substation Transformer Site, (h) Biratchowk Substation Control Building Site, (i) Biratchowk Substation Transformer Site, (j) Devdaha Substation Control Building Site, (k) Devdaha Substation Transformer Site, (l) Phikal Substation Control Building Site, (m) Phikal Substation Transformer Site, (n) Jitpur Substation Control Building Site, (o) Jitpur Substation Transformer Site, (p) Katari Substation Transformer Site, (q) Milan Chowk Substation Control Building Site, (r) Milan Chowk Substation, (s) Mukundapur Substation Control Building Site, (t) Mukundapur Substation Transformer Site, (u) Rangeli Substation Control Building Site, (v) Rangeli Substation Transformer Site, (w) Simrourgadha Substation Control Building Site, (x) Simrourgadha Substation Transformer site, (y) Yedukuwa Substation Control Building Site, and (z) Yedukuwa Substation Transformer Site

Foundation characteristics of the soils of different parts of Nepal

(c)

Aurahi Sub Station Transformer Site

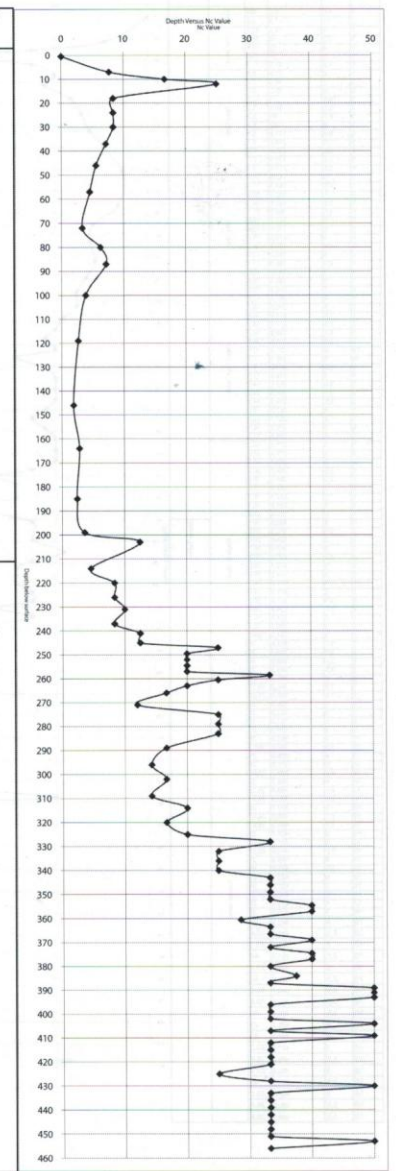
| SPA Test Data | | | | Lithology | Remarks |
|---------------|-------|-------------|------|-----------------------|---------|
| No. of Blows | Depth | Depth Below | Nc | | |
| 0 | 0.1 | 0.1 | 0.0 | Gravel, sand and silt | Filling |
| 10 | 6 | 6.1 | 16.7 | | |
| 10 | 4.5 | 10.6 | 22.2 | | |
| 10 | 4 | 14.6 | 25.0 | | |
| 10 | 3.5 | 18.1 | 28.6 | | |
| 10 | 3.5 | 21.6 | 28.6 | | |
| 10 | 4 | 25.6 | 25.0 | | |
| 10 | 3.5 | 29.1 | 28.6 | | |
| 10 | 4 | 33.1 | 25.0 | | |
| 10 | 4.5 | 37.6 | 22.2 | | |
| 10 | 4 | 41.6 | 25.0 | | |
| 10 | 4.5 | 46.1 | 22.2 | | |
| 10 | 4 | 50.1 | 25.0 | | |
| 10 | 5 | 55.1 | 20.0 | | |
| 10 | 7.5 | 62.6 | 13.3 | | |
| 5 | 4 | 66.6 | 12.5 | | |
| 5 | 2.5 | 69.1 | 20.0 | | |
| 5 | 3 | 72.1 | 16.7 | | |
| 5 | 4 | 76.1 | 12.5 | | |
| 5 | 5.5 | 81.6 | 9.1 | | |
| 5 | 6 | 87.6 | 8.3 | | |
| 5 | 6.5 | 94.1 | 7.7 | | |
| 5 | 6.5 | 100.6 | 7.7 | | |
| 5 | 9 | 109.6 | 5.6 | | |
| 5 | 4.5 | 114.1 | 11.1 | | |
| 10 | 23.5 | 137.6 | 4.3 | | |
| 5 | 16.5 | 154.1 | 3.0 | | |
| 5 | 12 | 166.1 | 4.2 | | |
| 5 | 9 | 175.1 | 5.6 | | |
| 5 | 5.5 | 180.6 | 9.1 | | |
| 5 | 6.5 | 187.1 | 7.7 | | |
| 5 | 5.5 | 192.6 | 9.1 | | |
| 5 | 6 | 198.6 | 8.3 | | |
| 5 | 6 | 204.6 | 8.3 | | |
| 5 | 5.5 | 210.1 | 9.1 | | |
| 5 | 4 | 214.1 | 12.5 | | |
| 5 | 3.5 | 217.6 | 14.3 | | |
| 5 | 4 | 221.6 | 12.5 | | |
| 5 | 3.5 | 225.1 | 14.3 | | |
| 5 | 3.5 | 228.6 | 14.3 | | |
| 5 | 2.5 | 231.1 | 20.0 | | |
| 10 | 5 | 236.1 | 20.0 | | |
| 10 | 4 | 240.1 | 25.0 | | |
| 10 | 4.5 | 244.6 | 22.2 | | |
| 10 | 4 | 248.6 | 25.0 | | |
| 10 | 3.5 | 252.1 | 28.6 | | |
| 10 | 3 | 255.1 | 33.3 | | |
| 10 | 4 | 259.1 | 25.0 | | |
| 10 | 3 | 262.1 | 33.3 | | |
| 10 | 3.5 | 265.6 | 28.6 | | |
| 10 | 3.5 | 269.1 | 28.6 | | |
| 10 | 2.5 | 271.6 | 40.0 | | |
| 10 | 3.5 | 275.1 | 28.6 | | |
| 10 | 3.5 | 278.6 | 28.6 | | |
| 10 | 3 | 281.6 | 33.3 | | |
| 10 | 3.5 | 285.1 | 28.6 | | |
| 10 | 3.5 | 288.6 | 28.6 | | |
| 10 | 2.5 | 291.1 | 40.0 | | |
| 10 | 2.5 | 293.6 | 40.0 | | |
| 10 | 4 | 297.6 | 25.0 | | |
| 10 | 3 | 300.6 | 33.3 | | |
| 10 | 4 | 304.6 | 25.0 | | |
| 10 | 3.5 | 308.1 | 28.6 | | |
| 10 | 1.5 | 309.6 | 66.7 | | |
| 10 | 1.5 | 311.1 | 66.7 | | |
| 10 | 2.5 | 313.6 | 40.0 | | |
| 10 | 4 | 317.6 | 25.0 | | |
| 10 | 2.5 | 320.1 | 40.0 | | |
| 10 | 3 | 323.1 | 33.3 | | |
| 10 | 3 | 326.1 | 33.3 | | |
| 10 | 3.5 | 329.6 | 28.6 | | |
| 10 | 3 | 332.6 | 33.3 | | |
| 10 | 3.5 | 336.1 | 28.6 | | |
| 10 | 1.5 | 337.6 | 66.7 | | |
| 10 | 2.5 | 340.1 | 40.0 | | |
| 10 | 2 | 342.1 | 50.0 | | |
| 10 | 3 | 345.1 | 33.3 | | |
| 10 | 3 | 348.1 | 33.3 | | |
| 10 | 3 | 351.1 | 33.3 | | |
| 10 | 3 | 354.1 | 33.3 | | |
| 10 | 3.5 | 357.6 | 28.6 | | |
| 10 | 2.5 | 360.1 | 40 | | |
| 10 | 2.5 | 362.6 | 40 | | |
| 10 | 3.5 | 366.1 | 28.6 | | |
| 10 | 2.5 | 368.6 | 40 | | |
| 10 | 3 | 371.6 | 33.3 | | |
| 10 | 3 | 374.6 | 33.3 | | |
| 10 | 3 | 377.6 | 33.3 | | |
| 10 | 3 | 380.6 | 33.3 | | |
| 10 | 2.5 | 383.1 | 40 | | |
| 10 | 2.5 | 385.6 | 40 | | |
| 10 | 2 | 387.6 | 50 | | |
| 10 | 2 | 389.6 | 50 | | |
| 10 | 3 | 392.6 | 33.3 | | |
| 10 | 2 | 394.6 | 50 | | |
| 10 | 3 | 397.6 | 33.3 | | |
| 10 | 1.5 | 399.1 | 66.7 | | |
| 10 | 2.5 | 401.6 | 40 | | |
| 10 | 2 | 403.6 | 50 | | |
| 10 | 2.5 | 406.1 | 40 | | |
| 15 | 3 | 409.1 | 50 | | |
| 10 | 2 | 411.1 | 50 | | |
| 10 | 1.5 | 412.6 | 66.7 | | |
| 10 | 2 | 414.6 | 50 | | |
| 10 | 2 | 416.6 | 50 | | |
| 10 | 2 | 418.6 | 50 | | |
| 10 | 2 | 420.6 | 50 | | |



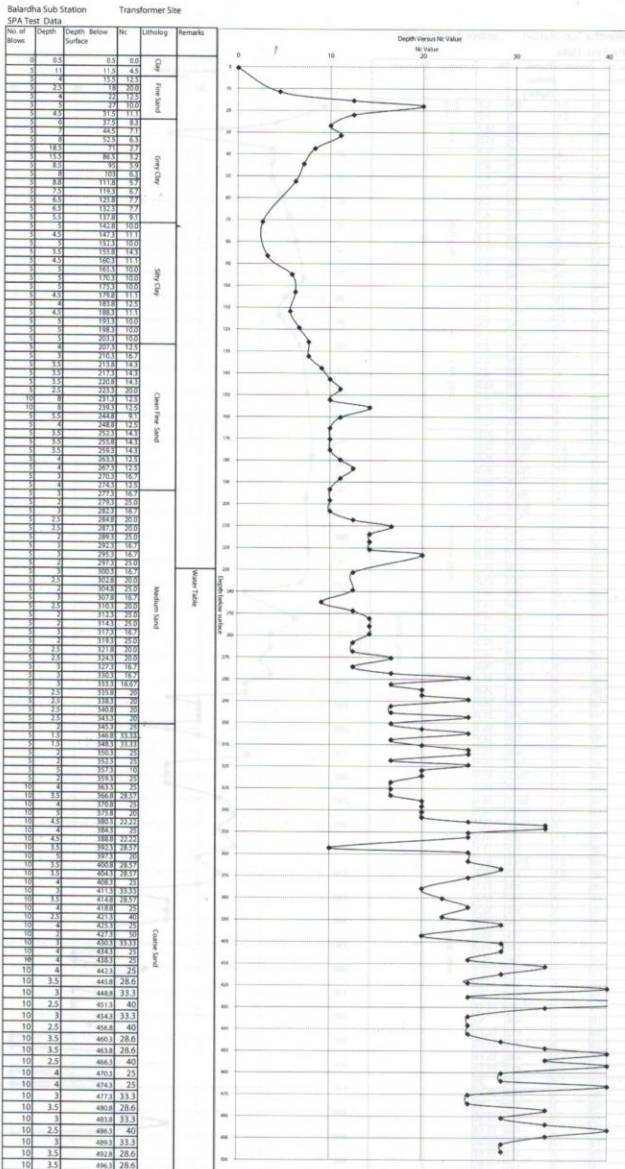
(d)

Balardha Sub Station Control Building Site

| SPA Test Data | | | | Lithology | Remarks |
|---------------|-------|---------------------|------|------------|---------|
| No. of Blows | Depth | Depth Below Surface | Nc | | |
| 0 | 0.5 | 0.5 | 0.0 | Silt Sand | |
| 5 | 6.5 | 7 | 7.7 | | |
| 5 | 3 | 10 | 16.7 | | |
| 5 | 2 | 12 | 25.0 | Silty Clay | |
| 5 | 6 | 18 | 8.3 | | |
| 5 | 6 | 24 | 8.3 | | |
| 5 | 6 | 30 | 8.3 | | |
| 5 | 7 | 37 | 7.1 | | |
| 5 | 9 | 46 | 5.6 | | |
| 5 | 11 | 57 | 4.5 | | |
| 5 | 13 | 72 | 3.3 | | |
| 5 | 8 | 80 | 6.3 | | |
| 5 | 7 | 87 | 7.1 | | |
| 5 | 13 | 100 | 3.3 | | |
| 5 | 19 | 119 | 2.6 | | |
| 5 | 27 | 146 | 1.9 | | |
| 5 | 18 | 164 | 2.8 | | |
| 5 | 21 | 185 | 2.4 | | |
| 5 | 14 | 199 | 3.6 | | |
| 5 | 4 | 203 | 12.5 | | |
| 5 | 11 | 214 | 4.3 | | |
| 5 | 6 | 220 | 8.3 | | |
| 5 | 6 | 226 | 8.3 | | |
| 5 | 5 | 231 | 10.0 | | |
| 5 | 6 | 237 | 8.3 | | |
| 5 | 4 | 241 | 12.5 | | |
| 5 | 4 | 243 | 12.5 | | |
| 5 | 2 | 247 | 25.0 | | |
| 5 | 2.5 | 249.5 | 20.0 | | |
| 5 | 2.5 | 252 | 20.0 | | |
| 5 | 2.5 | 254.5 | 20.0 | | |
| 5 | 2.5 | 257 | 20.0 | | |
| 5 | 1.5 | 258.5 | 33.3 | | |
| 5 | 2 | 260.5 | 25.0 | | |
| 5 | 2.5 | 263 | 20.0 | | |
| 5 | 3 | 266 | 16.7 | | |
| 6 | 5 | 271 | 12.0 | | |
| 10 | 4 | 275 | 25.0 | | |
| 10 | 4 | 279 | 25.0 | | |
| 10 | 4 | 283 | 25.0 | | |
| 10 | 6 | 289 | 16.7 | | |
| 10 | 7 | 296 | 14.3 | | |
| 10 | 6 | 302 | 16.7 | | |
| 10 | 7 | 309 | 14.3 | | |
| 10 | 5 | 314 | 20.0 | | |
| 10 | 6 | 320 | 16.7 | | |
| 10 | 5 | 325 | 20.0 | | |
| 10 | 3 | 328 | 33.3 | | |
| 10 | 4 | 332 | 25.0 | | |
| 10 | 4 | 336 | 25.0 | | |
| 10 | 4 | 340 | 25.0 | | |
| 10 | 3 | 343 | 33.3 | | |
| 10 | 3 | 346 | 33.3 | | |
| 10 | 3 | 349 | 33.3 | | |
| 10 | 3 | 352 | 33.3 | | |
| 10 | 2.5 | 354.5 | 40.0 | | |
| 10 | 2.5 | 357 | 40.0 | | |
| 10 | 3.5 | 360.5 | 28.6 | | |
| 10 | 3 | 363.5 | 33.3 | | |
| 10 | 2.5 | 366.5 | 33.3 | | |
| 10 | 2.5 | 369 | 40.0 | | |
| 10 | 3 | 372 | 33.3 | | |
| 10 | 2.5 | 374.5 | 40.0 | | |
| 10 | 2.5 | 377 | 40.0 | | |
| 10 | 4 | 380 | 33.3 | | |
| 15 | 4 | 384 | 37.5 | | |
| 10 | 3 | 387 | 33.3 | | |
| 10 | 2 | 389 | 50.0 | | |
| 10 | 2 | 391 | 50.0 | | |
| 10 | 2 | 393 | 50.0 | | |
| 10 | 3 | 396 | 33.3 | | |
| 10 | 3 | 399 | 33.3 | | |
| 10 | 3 | 402 | 33.3 | | |
| 10 | 2 | 404 | 50 | | |
| 10 | 3 | 407 | 33.3 | | |
| 10 | 2 | 409 | 50 | | |
| 10 | 3 | 412 | 33.3 | | |
| 10 | 3 | 415 | 33.3 | | |
| 10 | 3 | 418 | 33.3 | | |
| 10 | 3 | 421 | 33.3 | | |
| 10 | 4 | 425 | 25 | | |
| 10 | 3 | 428 | 33.3 | | |
| 10 | 2 | 430 | 50 | | |
| 10 | 3 | 433 | 33.3 | | |
| 10 | 3 | 436 | 33.3 | | |
| 10 | 3 | 439 | 33.3 | | |
| 10 | 3 | 442 | 33.3 | | |
| 10 | 3 | 445 | 33.3 | | |
| 10 | 3 | 448 | 33.3 | | |
| 10 | 3 | 451 | 33.3 | | |
| 10 | 2 | 453 | 50 | | |
| 10 | 3 | 456 | 33.3 | | |

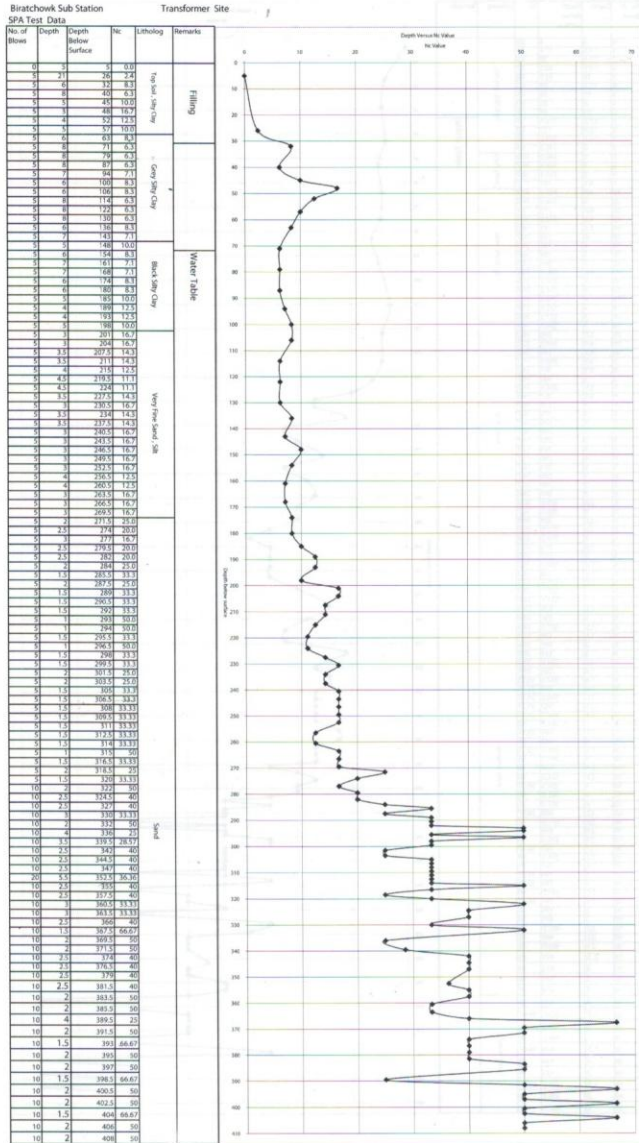


(e)



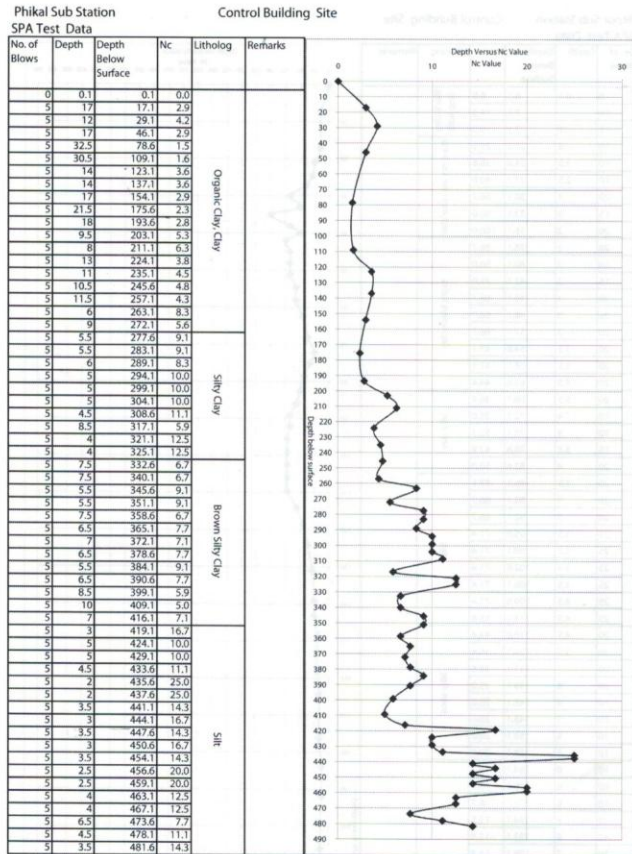
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(j)

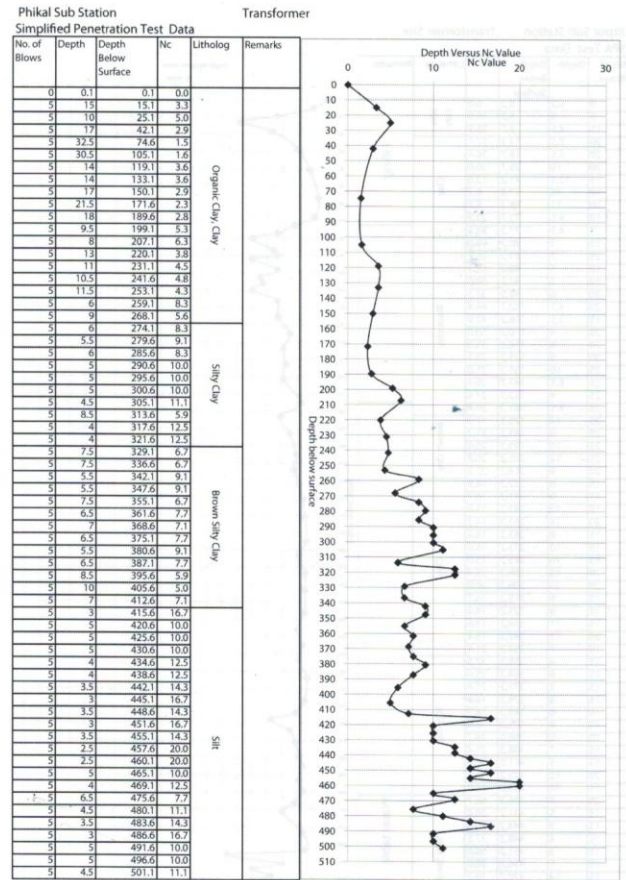


Foundation characteristics of the soils of different parts of Nepal

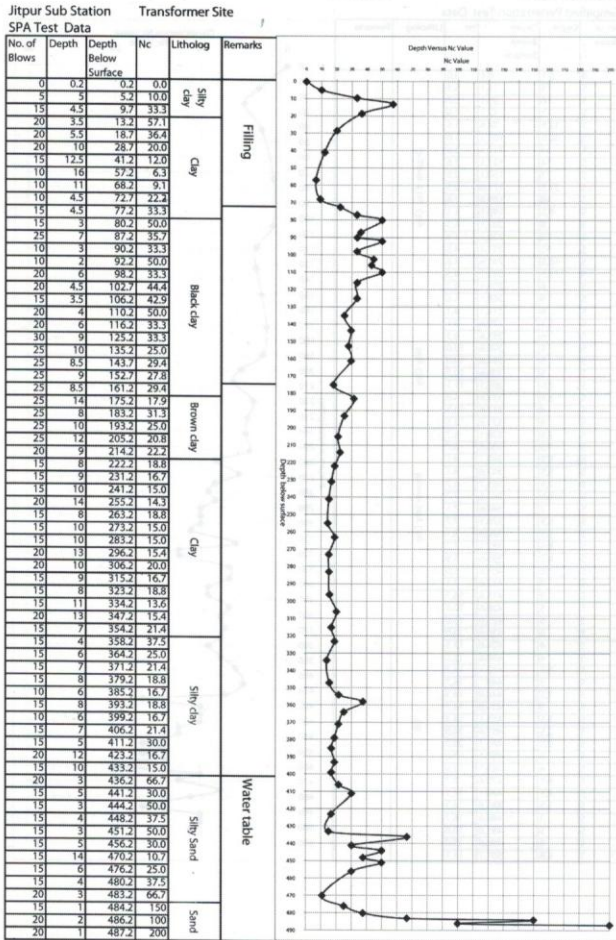
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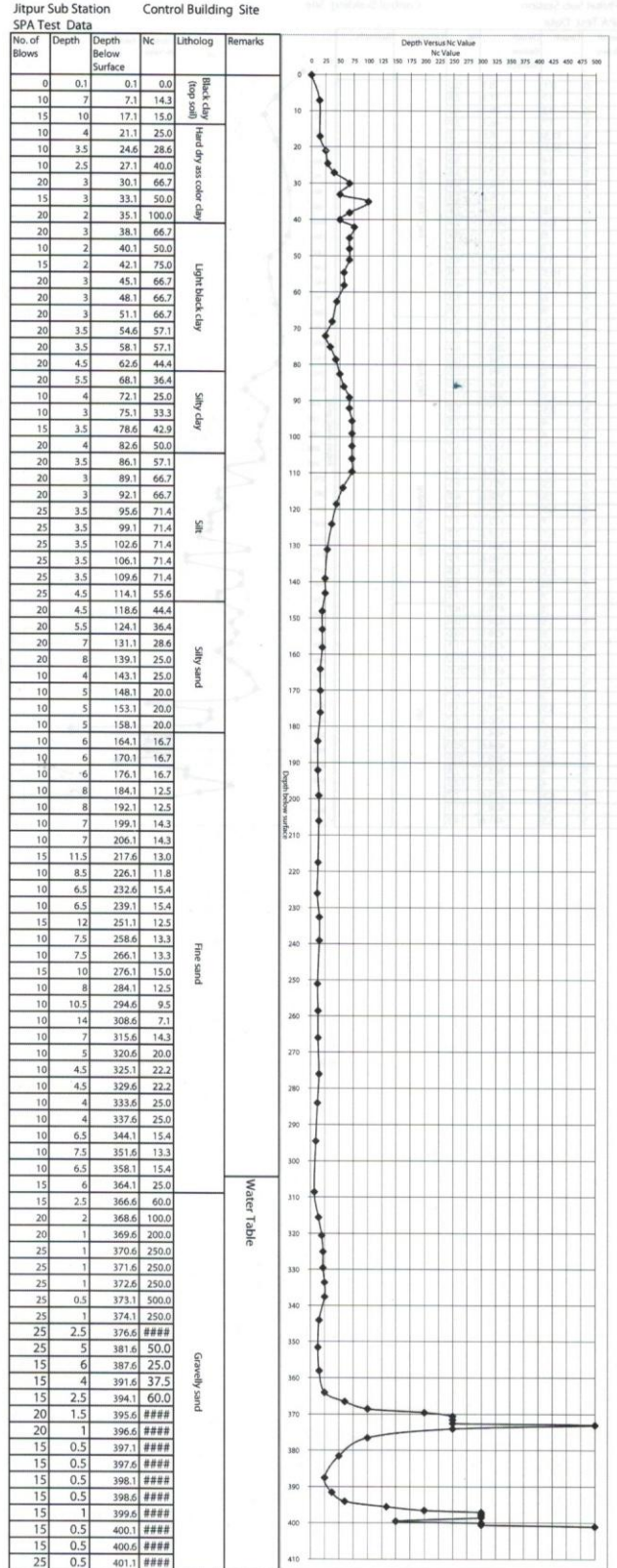
(m)



(n)

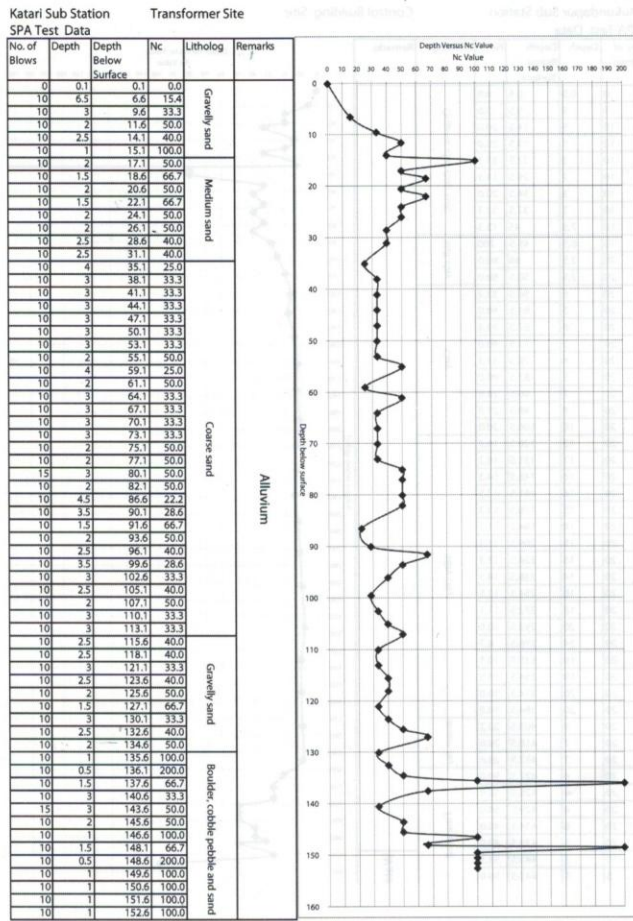


(m) (o)

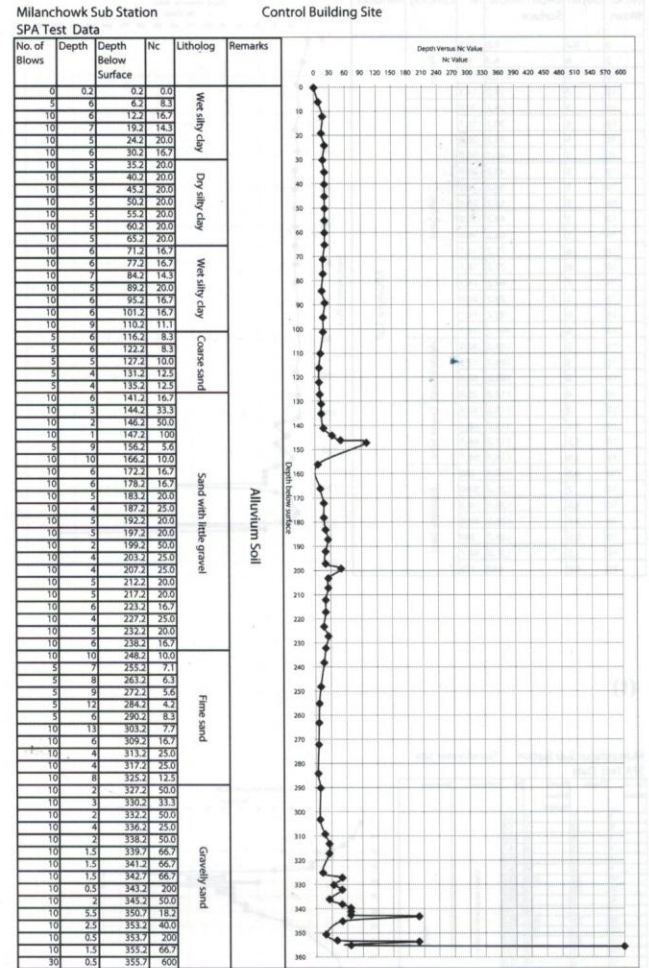


Foundation characteristics of the soils of different parts of Nepal

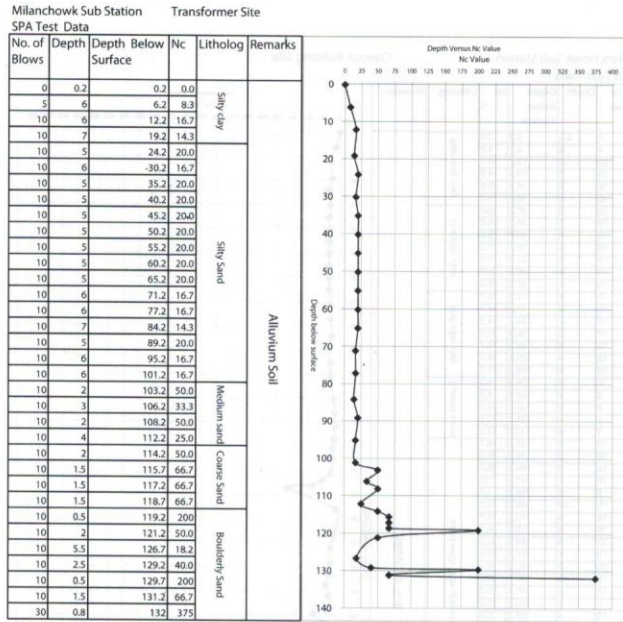
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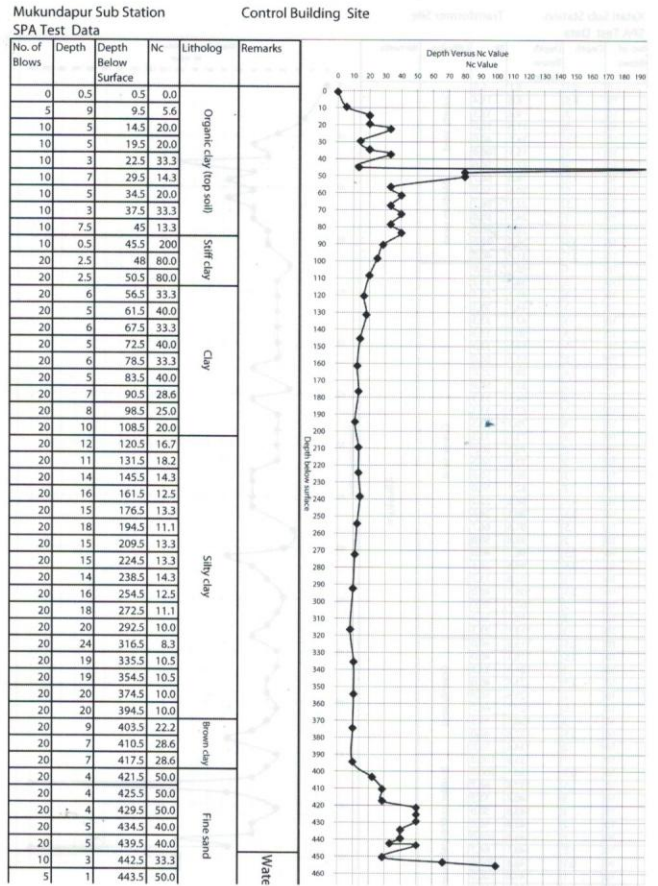
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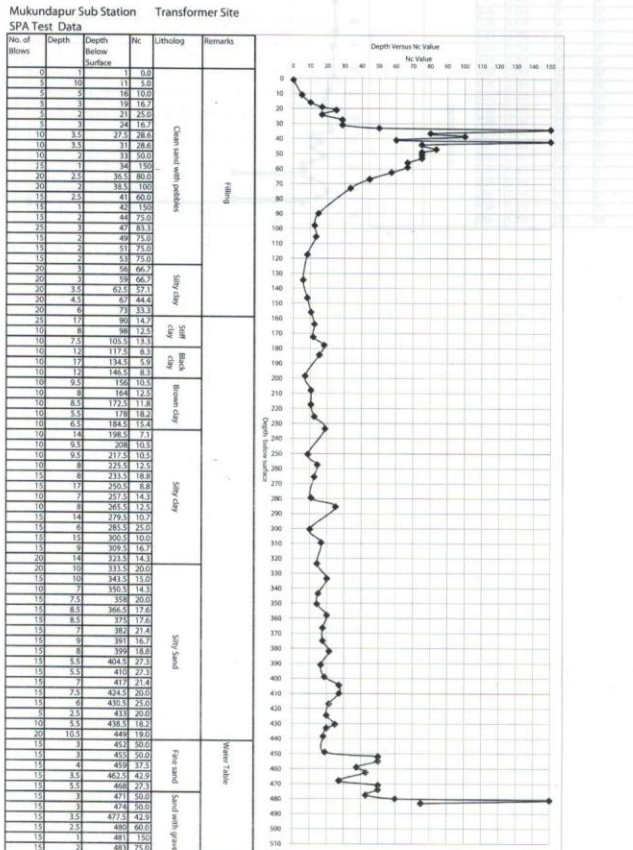
(r)



(s)



(t)

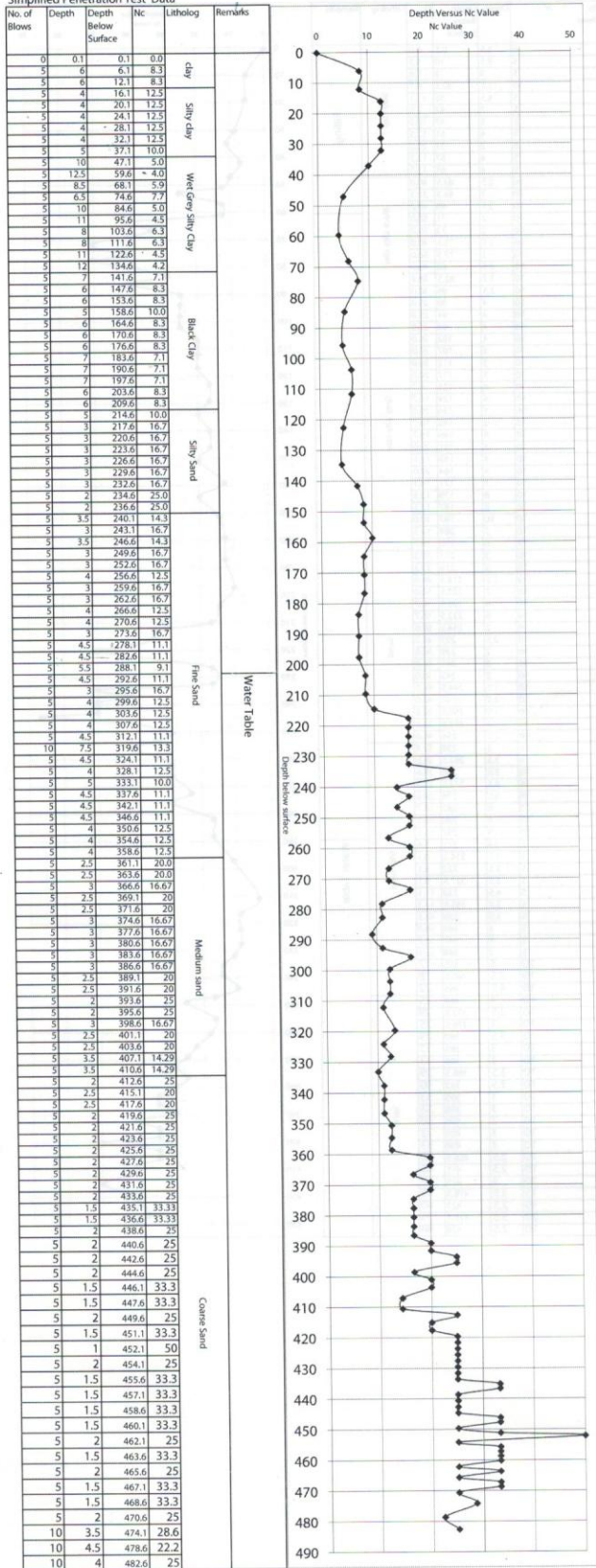


Foundation characteristics of the soils of different parts of Nepal

(u)

Rangeli Sub Station Control Building Site

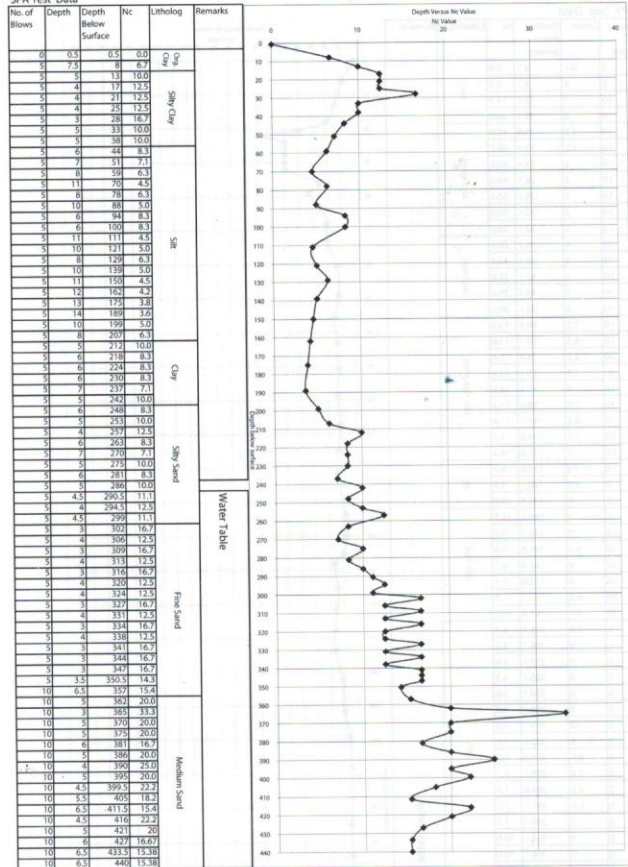
Simplified Penetration Test Data



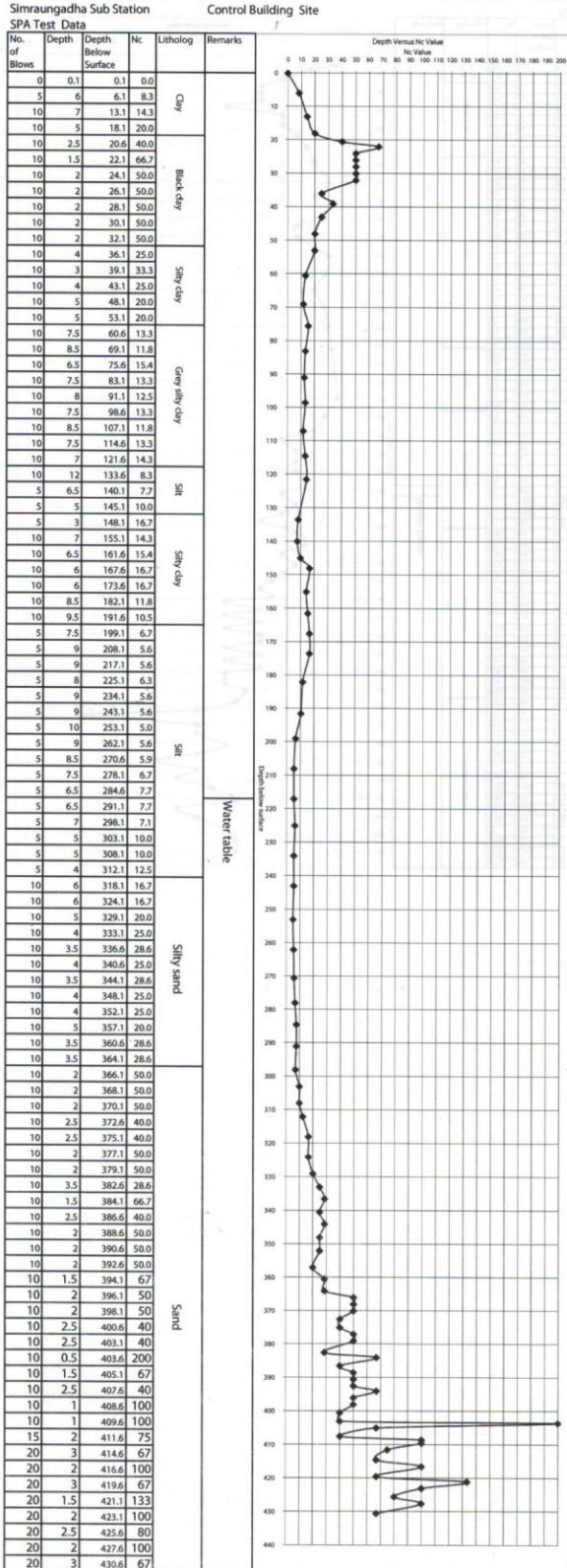
(v)

Rangeli Sub Station Transformer Site

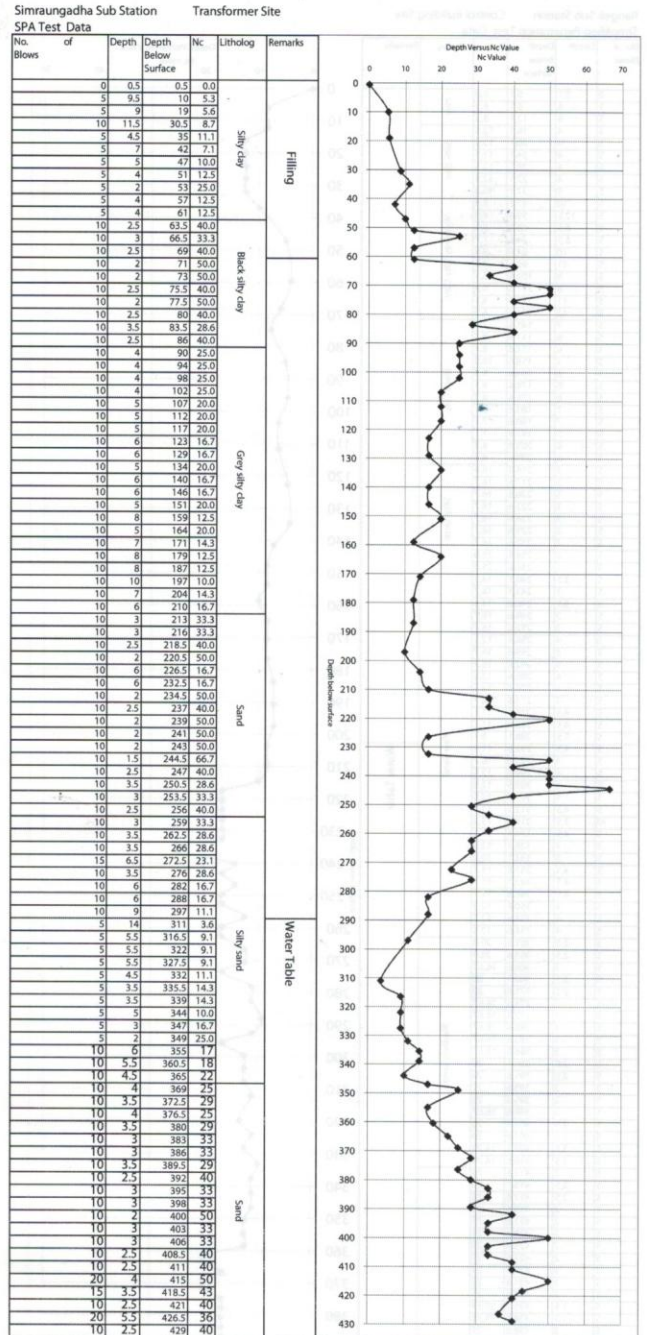
SPA Test Data



(w)

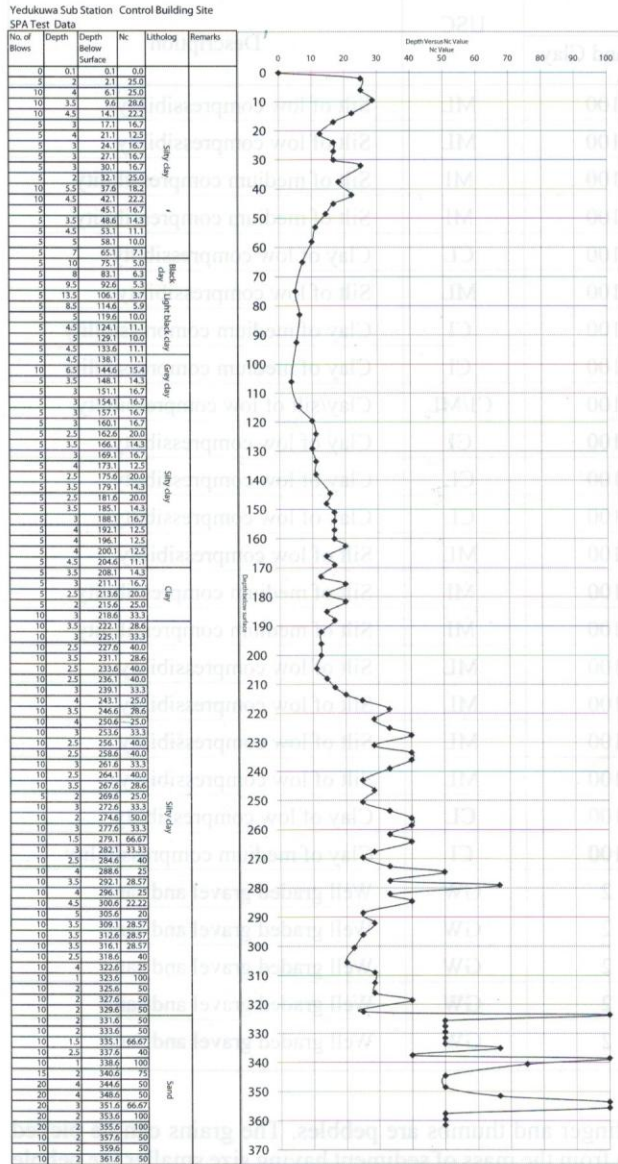


(x)



Foundation characteristics of the soils of different parts of Nepal

(y)



(z)

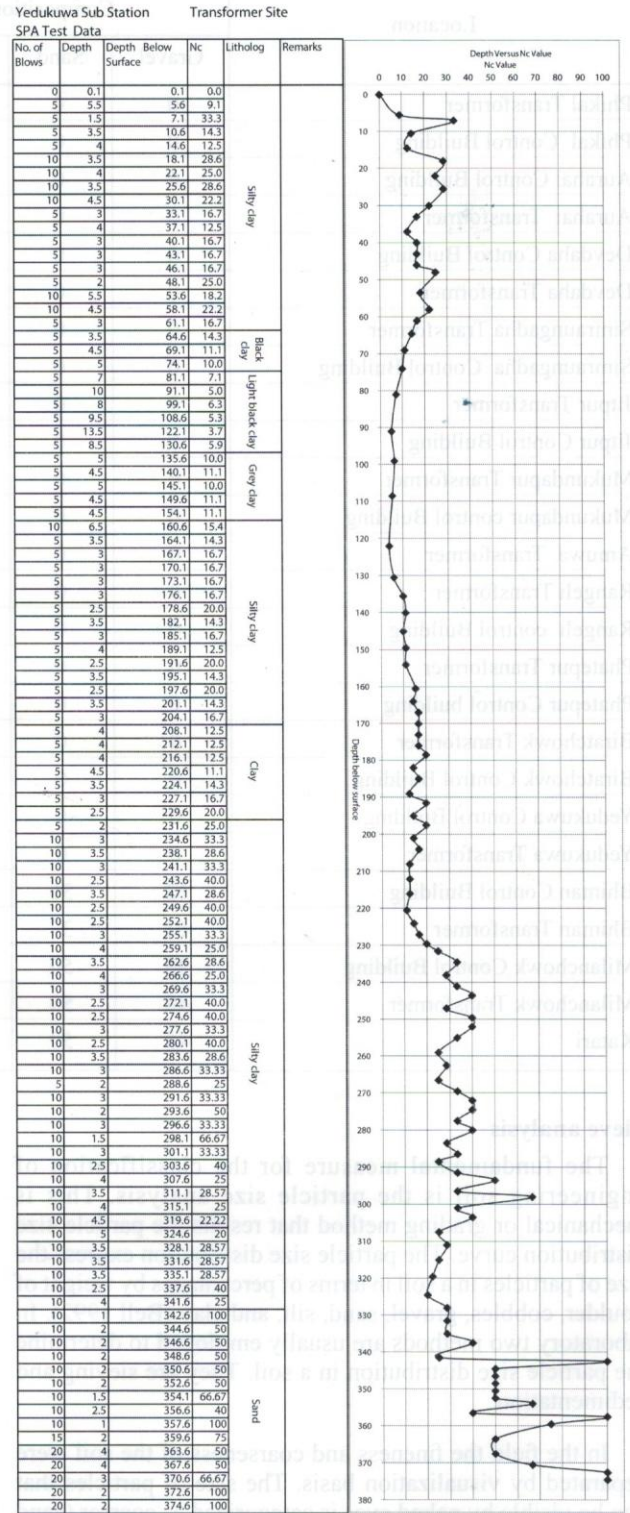


Table 2: Results of the sieve analysis

| Location | Composition % | | | USC | Description |
|-------------------------------|---------------|------|---------------|-------|----------------------------------|
| | Gravel | Sand | Silt and Clay | | |
| Phikal Transformer | 0 | 0 | 100 | ML | Silt of low compressibility |
| Phikal Control Building | 0 | 0 | 100 | ML | Silt of low compressibility |
| Aurahi Control Building | 0 | 0 | 100 | MI | Silt of medium compressibility |
| Aurahi Transformer | 0 | 0 | 100 | MI | Silt of medium compressibility |
| Devdaha Control Building | 0 | 0 | 100 | CL | Clay of low compressibility |
| Devdaha Transformer | 0 | 0 | 100 | ML | Silt of low compressibility |
| Simraungadha Transformer | 0 | 0 | 100 | CI | Clay of medium compressibility |
| Simraungadha Control Building | 0 | 0 | 100 | CI | Clay of medium compressibility |
| Jitpur Transformer | 0 | 0 | 100 | CI/ML | Clay/silt of low compressibility |
| Jitpur Control Building | 0 | 0 | 100 | CI | Clay of low compressibility |
| Mukundapur Transformer | 0 | 0 | 100 | CL | Clay of low compressibility |
| Mukundapur control Building | 0 | 0 | 100 | CL | Clay of low compressibility |
| Amuwa Transformer | 0 | 0 | 100 | ML | Silt of low compressibility |
| Rangeli Transformer | 0 | 0 | 100 | MI | Silt of medium compressibility |
| Rangeli control Building | 0 | 0 | 100 | MI | Silt of medium compressibility |
| Phatepur Transformer | 0 | 0 | 100 | ML | Silt of low compressibility |
| Phatepur Control building | 0 | 0 | 100 | ML | Silt of low compressibility |
| Biratchowk Transformer | 0 | 0 | 100 | ML | Silt of low compressibility |
| Biratchowk Control Building | 0 | 0 | 100 | ML | Silt of low compressibility |
| Yedukuwa Control Building | 0 | 0 | 100 | CL | Clay of low compressibility |
| Yedukuwa Transformer | 0 | 0 | 100 | CI | Clay of medium compressibility |
| Bhiman Control Building | 62 | 36 | 2 | GW | Well graded gravel and sand |
| Bhiman Transformer | 61 | 37 | 2 | GW | Well graded gravel and sand |
| Milanchowk Control Building | 48 | 50 | 2 | GW | Well graded gravel and sand |
| Milanchowk Transformer | 48 | 50 | 2 | GW | Well graded gravel and sand |
| Katari | 48 | 50 | 2 | GW | Well graded gravel and sand |

Sieve analysis

The fundamental measure for the classification of engineering soil is the particle size analysis. This is mechanical or grading method that results the particle size distribution curve. The particle size distribution express the size of particles in a soil in terms of percentages by weight of boulder, cobbles, gravel, sand, silt, and clay (Bell 1992). In laboratory two methods are usually employed to determine the particle size distribution in a soil. They are sieving and sedimentation.

In the field the fineness and coarseness of the soil were separated by visualization basis. The size of particles that can be visible by naked eyes is categorized as coarser (sand and gravel) otherwise as finer (Silt and clays). Among the coarser particles the grain that cannot be held in a single paw is considered as boulder. The grains can be easily grasped within a hand are cobbles and those that can be held between

a finger and thumbs are pebbles. The grains can be picked up from the mass of sediment having size smaller the pebble is considered as granules. The smaller grains than that of granules are considered as sands. The finer particles of silt and clays are determined by chewing a small fraction of soil. During chewing if it feels smooth the soil is considered as clay and if feels gritty, the soil is considered as silt.

According to the distribution of particle size, the soil can be classified as following:

Uniformly graded: the proportion of particle size concentrating within a narrow range is described as uniformly or poorly graded.

Well graded: Coarse-grained soil without excess of particles in any size range and without any lack of their intermediate size-range is regarded as well-graded soil. A well graded soil posses a wide range of particle size ranging

Table 3: Results of the Atterberg limit test

| Location | Liquid Limit | Plastic Limit (PL) | Plasticity Index (PI) | A-line (IP) | Flow (FI) | Index Index (TI) | Toughness Phikal |
|-------------------------------|--------------|--------------------|-----------------------|-------------|-----------|------------------|------------------|
| Transformer | 28.5 | 23.61 | 4.89 | 6.21 | 8.38 | 0.58 | |
| Phikal Control Building | 34.4 | 28.97 | 5.43 | 10.51 | 3.63 | 1.50 | |
| Aurahi Control Building | 35 | 26.46 | 8.54 | 10.95 | 1.86 | 4.59 | |
| Aurahi Transformer | 37 | 27.3 | 9.70 | 12.41 | 1.61 | 6.02 | |
| Devdaha Control Building | 22.2 | 12.22 | 9.98 | 1.6 | 1.79 | 5.59 | |
| Devdaha Transformer | 22 | 18.32 | 3.68 | 1.46 | 3.36 | 1.09 | |
| Simraungadha Transformer | 44 | 21.94 | 22.06 | 17.52 | 1.94 | 11.39 | |
| Simraungadha Control Building | 44.4 | 21.94 | 22.46 | 17.81 | 1.8 | 12.47 | |
| Jitpur Transformer | 21.6 | 5.72 | 15.88 | 1.17 | 3.86 | 4.12 | |
| Jitpur Control Building | 20 | 4.46 | 15.34 | 0 | 1.12 | 13.68 | |
| Mukundapur Transformer | 30 | 13.86 | 16.14 | 7.3 | 1.47 | 10.96 | |
| Mukundapur control Building | 29.6 | 12.22 | 17.38 | 7.01 | 1.3 | 13.38 | |
| Amuwa Transformer | 18 | NA | NA | -1.46 | -0.18 | NA | |
| Rangeli Transformer | 46 | 42.42 | 3.58 | 18.98 | 2.94 | 1.22 | |
| Rangeli control Building | 48 | 46.67 | 1.33 | 20.44 | 2.07 | 0.64 | |
| Phatepur Transformer | 34 | 24.1 | 9.9 | 10.22 | -2.51 | -3.95 | |
| Phatepur Control building | 31.8 | 26.67 | 5.13 | 8.61 | 1.38 | 3.71 | |
| Biratchowk Transformer | 30 | 28.52 | 1.48 | 7.3 | 3.03 | 0.49 | |
| Biratchowk Control Building | 30.00 | 28.54 | 1.46 | 7.30 | 2.12 | 0.69 | |
| Yedukuwa Control Building | 24 | 13.33 | 10.67 | 2.92 | 2.66 | 4.02 | |
| Yedukuwa Transformer | 23 | 18.89 | 4.11 | 2.19 | 3.04 | 1.35 | |

from gravel to clay.

Gap or skip graded: A gap-graded soil lacks some or at least one intermediate size particles.

Sieve analyses of all the samples collected during the field work are presented in Table 2. The results of the sieve analysis show that the soil types found in the different sites are of medium to low compressibility i.e. excellent for the foundations.

Liquid limit and plastic limit test

The amount of water content in a cohesive soil controls the deformation behavior of that soil. This response of soil to the stress is regarded as the soil consistency. In other words soil consistency indicates the degree of firmness of a cohesive soil. A cohesive soil changes from non-plastic to plastic and plastic to viscous when the water content increases (Johnson and DeGraff 1988). The consistency for a particular soil is defined by the water content present in it when it changes its response to stress. These changes are called the Atterberg limits. The Atterberg limits are defined in terms of liquid limit (LL), plastic limit (PL), and shrinkage limit (SL).

Liquid limit (LL)

The water content at which a cohesive soil changes from viscous to plastic state and vice versa is regarded as the liquid limit. At this stage all soils possess certain small shear strength.

Plastic limit (PL)

The water content at the transition of a plastic soil into a semisolid soil is regarded as the plastic limit. At this limit the soil rolled into threads of about 3mm diameter just crumbles. Between the plastic limit and the liquid limit, the soil behaves as plastic material. This range of water content is termed as the Plasticity Index (PI) and is calculated by:

$$PI = LL - PL$$

Shrinkage limit (SL)

When the water content decreases from the plastic limit, the soil approaches a point where no volume of the soil change on further drying up. Beyond this point discoloration of soil may occur. This limit is called as shrinkage limit. Between the shrinkage limit and plastic limit the soil is considered as semisolid.

The results of the liquid limit and plastic limit are presented in Table 3. The results are used to find out Plasticity Index (PI), A-Line I_p , Flow Index and Toughness Index. They can be used also for the liquefaction potential for the soil (Lambe, 1951). Most of the sites are safe from liquefaction from the vertical loading except Biratchowk. Due to very shallow ground water table at Biratchowk sand boiling may occur and should be considered during detail design stage. The liquid limit and plastic limit data are used for the classification of the soil. These data are also used to cross check the strength parameters such as c (cohesion), ϕ (friction angle) and for determining the compressibility of the soil.

Moisture content

The results of moisture content are presented in Table 4. Moisture content is determined taking some portion of the undisturbed sample collected for the direct shear at the time of sample preparation for direct shear test and also from the disturbed sample. Both of these samples were waxed and sealed to prevent moisture loss. Generally the granular soil

Table 4: Results of the moisture contents

| Location | Moisture Content % |
|-------------------------------|--------------------|
| Phikal Transformer | 32.34 |
| Phikal Control Building | 31.97 |
| Aurahi Control Building | 28.85 |
| Aurahi Transformer | 28.98 |
| Devdaha Control Building | 18.43 |
| Devdaha Transformer | 17.15 |
| Simraungadha Transformer | 30.43 |
| Simraungadha Control Building | 31.05 |
| Jitpur Transformer | 12.11 |
| Jitpur Control Building | 10.78 |
| Mukundapur Transformer | 21.19 |
| Mukundapur control Building | 22.12 |
| Amuwa Transformer | 6.52 |
| Rangeli Transformer | 33.18 |
| Rangeli control Building | 34.32 |
| Phatepur Transformer | 36.44 |
| Phatepur Control building | 34.02 |
| Biratchowk Transformer | 31.62 |
| Biratchowk Control Building | 30.44 |
| Yedukuwa Control Building | 22.17 |
| Yedukuwa Transformer | 22.86 |
| Bhiman Control Building | 4.13 |
| Bhiman Transformer | 3.98 |
| Milanchowk Control Building | 1.32 |
| Milanchowk Transformer | 1.41 |
| Katari | 9.4 |

has less moisture content than the fine soil. The dry unit weight of the soil is considered as its unit weight of the soil for the calculation of the much reliable safe bearing capacity.

Specific gravity determination

Specific gravity is the ratio of the mass/ weight in the air of a given volume of dry soil material to the mass / weight of equal volume of distilled water at 4°C. But as the value of the specific gravity depends on the temperature, hence its value is reported at standard temperature of 27°C. Specific gravity of the soil passing through 4.75 mm sieve is considered here. Thus the specific gravity of the soil classified as GW is not considered here. The specific gravity of the soil is presented in Table 5.

Direct shear test

Direct shear test is carried to determine the shear strength parameters for a given soil. The strength of a soil depends of its resistance to shearing stresses. It is made up of basically the components; (1) Frictional – due to friction between individual particles and (2) Cohesive - due to adhesion between the soil particles. The test is carried out on either undisturbed samples or remolded samples. To facilitate the remolding purpose, a soil sample may be compacted at optimum moisture content in a compaction mould. Then

Table 5: Results of the specific gravity test

| Location | Specific Gravity |
|-------------------------------|------------------|
| Phikal Transformer Site | 2.64 |
| Phikal Control Building Site | 2.63 |
| Aurahi Control Building | 2.68 |
| Aurahi Transformer | 2.72 |
| Devdaha Control Building | 2.62 |
| Devdaha Transformer | 2.66 |
| Simraungadha Transformer | 2.74 |
| Simraungadha Control Building | 2.73 |
| Jitpur Transformer | 2.71 |
| Jitpur Control Building | 2.73 |
| Mukundapur Transformer | 2.70 |
| Mukundapur control Building | 2.78 |
| Amuwa Transformer Site | 2.64 |
| Rangeli Transformer | 2.71 |
| Rangeli control Building | 2.72 |
| Phatepur Transformer | 2.78 |
| Phatepur Control building | 2.76 |
| Biratchowk Transformer | 2.68 |
| Biratchowk Control Building | 2.72 |
| Yedukuwa Control Building | 2.67 |
| Yedukuwa Transformer | 2.72 |

specimen for the direct shear test could be obtained using the correct cutter provided.

A normal load is applied to the specimen and the specimen is sheared across the pre-determined horizontal plane between the two halves of the shear box. Measurements of shear load, shear displacement and normal displacement are recorded. The test is repeated for two or more identical specimens under different normal loads. From the results, the shear strength parameters can be determined. Here we use direct shear test to calculate the cohesion (C), friction angle (ϕ) and unit weight (γ) of the soil. The entire tests are carried out in undisturbed soil samples. These values are tabulated in Table 6.

Bearing capacity of the soil

The direct shear test data are used to calculate the

bearing capacity of the soil. The maximum allowable net loading intensity on the soil allowing for both shear and settlement effects is the allowable bearing capacity of the soil (Meyerhof, 1951). Shear parameters in the plain strain measured in the laboratory by the direct shear test are used for the calculations. Ultimate bearing capacity for strip foundation is calculated by using the ‘‘Terzaghi Equation’’ (Terzaghi and Peck, 1967) as given;

$$q = c N_c + \gamma z N_q + 0.5 \gamma B N_\gamma$$

Where q = ultimate bearing capacity in kN/m², c = cohesion in kN/m², N_c , N_q and N_γ are the Terzaghi’s bearing capacity coefficients, B = width of the foundation, Z = depth below the surface.

Bearing capacity of the soil is also calculated by using the ‘‘Hasen Equation’’ (Hansen 1970).

Table 6: Results of the direct shear test

| Sub Stations | Site | Dry Unit Weight (γ) | Angle of Internal Friction (ϕ) | Cohesion (C) |
|--------------|------------------|---------------------------------|--|-----------------|
| Phikal | Transformer | 13.06 | 28.5 | 5 |
| | Control Building | 14.10 | 30 | 6 |
| Aurahai | Transformer | 16.00 | 27 | 7 |
| | Control Building | 14.14 | 26 | 4 |
| DevDaha | Transformer | 15.30 | 28 | 1 |
| | Control Building | 14.95 | 27 | 1 |
| Simraungadha | Transformer | 15.84 | 22 | 6 |
| | Control Building | 16.20 | 28 | 11 |
| Jitpur | Transformer | 14.08 | 28 | 0 |
| | Control Building | 14.00 | 27 | 5 |
| Mukundapur | Transformer | 15.04 | 25 | 5 |
| | Control Building | 15.01 | 25.8 | 8 |
| Amuwa | Transformer | 15.21 | 25 | 2 |
| | Control Building | | | |
| Rangeli | Transformer | 13.51 | 21 | 14 |
| | Control Building | 14.45 | 26.9 | 8 |
| Phatepur | Transformer | 13.97 | 29.5 | 2 |
| | Control Building | 14.20 | 28.7 | 10 |
| Biratchowk | Transformer | 15.08 | 29 | 6 |
| | Control Building | 15.02 | 30 | 4 |
| Yedukuwa | Transformer | 16.20 | 31.2 | 9 |
| | Control Building | 15.92 | 30.5 | 7 |
| Bhiman | Transformer | 19.7 | 34.5 | 0 |
| | Control Building | 20.10 | 36 | 0 |
| MilanChowk | Transformer | 19.23 | 33.2 | 0 |
| | Control Building | 20.12 | 34.9 | 0 |
| Katari | Transformer | 18.89 | 35.8 | 0 |
| | Control Building | | | |

“Hansen Equation” is given as

$$q = c N_c Sc Zc + \gamma Z N_q Sq Zq + 0.5\gamma B N_\gamma S_\gamma Z_\gamma$$

Where $N_c = (N_q - 1) \cot\phi$

$N_q = \tan^2 (45 + \phi/2) e^{\gamma \tan\phi}$

$N_\gamma = 1.5 (N_q - 1) \tan\phi$ are the bearing capacity coefficients.

Sc, Sq and S_γ are the shape factors as follows;

$$Sc = 1 + (N_q B / N_c L)$$

$$Sq = 1 + \{(B / L) \tan\phi\}$$

$$S_\gamma = 1 - (0.4B / L)$$

Zc, Zq and Z_γ are the depth factors as follows;

$$Zc = 1 + (0.42 / B), Zq = 1 + \{2 \tan\phi (1 - \sin\phi) 2Z / B\}, Z_\gamma = 1$$

Z = depth of the foundation.

W = width of the foundation.

L = Length of the foundation.

Safe bearing capacity is calculated as

$$\text{Safe bearing capacity} = \{(q - \gamma Z) / F.S.\} + \gamma Z$$

Where q = bearing capacity, F.S. = Factor of safety, γ = Unit weight of soil., Z = depth of foundation.

The safe bearing capacity in general for the strip 1.2m wide, isolated 2m x 2m, Raft 5m x 5m for all the sites are calculated. The results of the bearing capacity using Terzaghi method are presented in Table 7. The results of the bearing capacity by Hasen method are presented in Table 8.

The inclination factor and the pore water pressure are not considered while calculating the bearing capacity of the soil. Similarly the bearing capacity by Terzaghi and Hansen methods depends on the friction angle and cohesion only

Table 7: Results of the bearing capacity by Terzagi Method, (a) Results of the bearing capacity for isolated footings (depth 2 m), (b) Results of the bearing capacity for the strip footings (depth 2 m), (c) Results of the bearing capacity for mat foundation (depth 2 m)

a. Calculation of the Bearing Capacity by Terzagi Method (Isolated Footings)

| Location | Ultimate Bearing Capacity (kN/m ²) | Safe Bearing Capacity (kN/m ²) | Net Safe Bearing Capacity (kN/m ²) |
|-------------------------------|--|--|--|
| Phikal Transformer Site | 937.48 | 338.61 | 286.37 |
| Phikal Control Building Site | 1135.47 | 406.69 | 350.29 |
| Aurahi Control Building | 673.60 | 252.81 | 196.25 |
| Aurahi Transformer | 907.00 | 334.33 | 270.33 |
| Devdaha Control Building | 656.90 | 248.87 | 189.07 |
| Devdaha Transformer | 797.00 | 296.27 | 235.07 |
| Simraungadha Transformer | 516.00 | 203.68 | 140.32 |
| Simraungadha Control Building | 1606.40 | 567.87 | 503.07 |
| Jitpur Transformer | 704.00 | 262.83 | 206.51 |
| Jitpur Control Building | 954.00 | 346.00 | 290.00 |
| Mukundapur Transformer | 653.40 | 247.88 | 187.72 |
| Mukundapur control Building | 1006.50 | 365.52 | 305.48 |
| Amuwa Transformer Site | 583.87 | 225.04 | 164.20 |
| Rangeli Transformer | 563.22 | 214.76 | 160.72 |
| Rangeli control Building | 1081.20 | 389.30 | 331.50 |
| Phatepur Transformer | 936.14 | 339.99 | 284.11 |
| Phatepur Control building | 1408.20 | 497.80 | 441.00 |
| Biratchowk Transformer | 1048.48 | 379.65 | 319.33 |
| Biratchowk Control Building | 2478.96 | 856.36 | 343.49 |
| Yedukuwa Control Building | 1570.76 | 555.43 | 491.75 |
| Yedukuwa Transformer | 2120.40 | 739.20 | 674.40 |
| Bhiman Control Building | 3778.80 | 1299.80 | 1219.40 |
| Bhiman Transformer | 3092.90 | 1070.37 | 991.57 |
| Milanchowk Control Building | 2857.04 | 992.59 | 912.11 |
| Milanchowk Transformer | 2596.05 | 903.81 | 826.89 |
| Katari | 3154.63 | 1089.32 | 1013.76 |

b. Calculation of the Bearing Capacity by Terzaghi Method (Strip Foundations)

| Location | Ultimate Bearing Capacity (kN/m ²) | Safe Bearing Capacity (kN/m ²) | Net Safe Bearing Capacity (kN/m ²) |
|-------------------------------|--|--|--|
| Phikal Transformer Site | 843.45 | 298.56 | 255.03 |
| Phikal Control Building Site | 1024.36 | 360.25 | 313.25 |
| Aurahi Control Building | 605.73 | 220.76 | 173.63 |
| Aurahi Transformer | 830.20 | 298.07 | 244.73 |
| Devdaha Control Building | 585.14 | 214.98 | 165.15 |
| Devdaha Transformer | 711.32 | 257.51 | 206.51 |
| Simraungadha Transformer | 471.65 | 178.34 | 125.54 |
| Simraungadha Control Building | 1463.84 | 509.55 | 455.55 |
| Jitpur Transformer | 625.15 | 227.16 | 180.22 |
| Jitpur Control Building | 864.40 | 306.80 | 260.13 |
| Mukundapur Transformer | 595.05 | 218.40 | 168.27 |
| Mukundapur control Building | 922.44 | 327.49 | 277.46 |
| Amuwa Transformer Site | 524.86 | 195.23 | 144.53 |
| Rangeli Transformer | 530.80 | 194.95 | 149.91 |
| Rangeli control Building | 988.72 | 348.84 | 300.67 |
| Phatepur Transformer | 835.56 | 297.15 | 250.58 |
| Phatepur Control building | 1288.92 | 448.57 | 401.24 |
| Biratchowk Transformer | 951.97 | 337.43 | 287.16 |
| Biratchowk Control Building | 2166.54 | 742.21 | 304.04 |
| Yedukuwa Control Building | 1417.93 | 493.87 | 440.80 |
| Yedukuwa Transformer | 1900.08 | 654.96 | 600.96 |
| Bhiman Control Building | 3232.08 | 1104.16 | 1037.16 |
| Bhiman Transformer | 2706.78 | 928.53 | 862.86 |
| Milanchowk Control Building | 2454.64 | 845.04 | 777.97 |
| Milanchowk Transformer | 2234.53 | 770.48 | 706.38 |
| Katari | 2708.83 | 928.13 | 865.16 |

c. Calculation of the Bearing Capacity by Terzaghi Method (Mat Foundations)

| Location | Ultimate Bearing Capacity (kN/m ²) | Safe Bearing Capacity (kN/m ²) | Net Safe Bearing Capacity (kN/m ²) |
|-------------------------------|--|--|--|
| Phikal Transformer Site | 1290.10 | 456.15 | 403.91 |
| Phikal Control Building Site | 1552.13 | 545.58 | 489.18 |
| Aurahai Control Building | 928.12 | 337.65 | 281.09 |
| Aurahai Transformer | 1195.00 | 430.33 | 366.33 |
| Devdaha Control Building | 926.00 | 338.57 | 278.77 |
| Devdaha Transformer | 1118.30 | 403.37 | 342.17 |
| Simraungadha Transformer | 682.32 | 259.12 | 195.76 |
| Simraungadha Control Building | 2141.00 | 746.07 | 681.27 |
| Jitpur Transformer | 999.68 | 361.39 | 305.07 |
| Jitpur Control Building | 1290.00 | 458.00 | 402.00 |
| Mukundapur Transformer | 872.24 | 320.83 | 260.67 |
| Mukundapur control Building | 1321.71 | 470.59 | 410.55 |
| Amuwa Transformer Site | 805.18 | 298.81 | 237.97 |
| Rangeli Transformer | 684.81 | 255.29 | 201.25 |
| Rangeli control Building | 1428.00 | 504.90 | 447.10 |
| Phatepur Transformer | 1313.33 | 465.72 | 409.84 |
| Phatepur Control building | 1855.50 | 646.90 | 590.10 |
| Biratchowk Transformer | 1410.40 | 500.29 | 439.97 |
| Biratchowk Control Building | 3650.52 | 1246.88 | 491.44 |
| Yedukuwa Control Building | 2143.88 | 746.47 | 682.79 |
| Yedukuwa Transformer | 2946.60 | 1014.60 | 949.80 |
| Bhiman Control Building | 5829.00 | 1983.20 | 1902.80 |
| Bhiman Transformer | 4540.85 | 1553.02 | 1474.22 |
| Milanchowk Control Building | 4366.04 | 1495.59 | 1415.11 |
| Milanchowk Transformer | 3951.77 | 1355.72 | 1278.80 |
| Katari | 4826.40 | 1646.58 | 1571.02 |

Table 8: Results of the bearing capacity by Hasen Method, (a) Results of the bearing capacity for isolated footings (depth 2 m), (b) Results of the bearing capacity for the strip footings (depth 2 m), (c) Results of the bearing capacity for mat foundation (depth 2 m)

| a. Calculation of the bearing capacity by Hasen method (Isolated Footings) | | | | | |
|---|--|--|--|--|--|
| Location | Ultimate Bearing Capacity (kN/m ²) | Safe Bearing Capacity (kN/m ²) | Net Safe Bearing Capacity (kN/m ²) | | |
| Phikal Transformer Site | 1202.37 | 418.20 | 374.67 | | |
| Phikal Control Building Site | 1590.02 | 548.81 | 501.81 | | |
| Aurahi Control Building | 910.58 | 322.38 | 275.25 | | |
| Aurahi Transformer | 1284.78 | 449.59 | 396.26 | | |
| Devdaha Control Building | 912.36 | 324.05 | 274.22 | | |
| Devdaha Transformer | 1053.61 | 371.60 | 320.60 | | |
| Simraungadha Transformer | 704.63 | 256.00 | 203.20 | | |
| Simraungadha Control Building | 2365.34 | 810.05 | 527.46 | | |
| Jitpur Transformer | 917.39 | 324.57 | 277.64 | | |
| Jitpur Control Building | 1344.18 | 466.73 | 327.23 | | |
| Mukundapur Transformer | 897.15 | 319.10 | 268.97 | | |
| Mukundapur control Building | 1431.84 | 497.29 | 343.66 | | |
| Amuwa Transformer Site | 773.00 | 277.95 | 227.25 | | |
| Rangeli Transformer | 824.85 | 292.96 | 247.93 | | |
| Rangeli control Building | 1563.50 | 540.43 | 382.37 | | |
| Phatepur Transformer | 1229.00 | 428.29 | 381.73 | | |
| Phatepur Control building | 2100.04 | 718.95 | 509.50 | | |
| Biratchowk Transformer | 1485.98 | 515.43 | 465.17 | | |
| Biratchowk Control Building | 3297.16 | 1119.08 | 480.38 | | |
| Yedukuwa Control Building | 2450.02 | 837.90 | 608.64 | | |
| Yedukuwa Transformer | 3003.66 | 1022.82 | 724.09 | | |
| Bhiman Control Building | 4990.24 | 1690.21 | 1209.90 | | |
| Bhiman Transformer | 3943.62 | 1340.81 | 956.48 | | |
| Milanchowk Control Building | 3754.04 | 1278.17 | 1034.09 | | |
| Milanchowk Transformer | 3346.39 | 1141.10 | 777.31 | | |
| Katari | 4059.90 | 1378.49 | 1104.67 | | |

b. Calculation of the bearing capacity by Hasen method (Strip Footings)

| Location | Ultimate Bearing Capacity (kN/m ²) | Safe Bearing Capacity (kN/m ²) | Net Safe Bearing Capacity (kN/m ²) |
|-------------------------------|--|--|--|
| Phikal Transformer Site | 1345.75 | 466.00 | 422.46 |
| Phikal Control Building Site | 1774.21 | 610.20 | 563.20 |
| Aurahi Control Building | 1022.32 | 359.63 | 312.49 |
| Aurahi Transformer | 1447.44 | 503.81 | 450.48 |
| Devdaha Control Building | 1009.47 | 356.42 | 306.59 |
| Devdaha Transformer | 1161.90 | 407.70 | 356.70 |
| Simraungadha Transformer | 801.73 | 288.36 | 235.56 |
| Simraungadha Control Building | 2654.52 | 906.44 | 852.44 |
| Jitpur Transformer | 1007.10 | 354.47 | 307.54 |
| Jitpur Control Building | 1500.49 | 518.83 | 472.16 |
| Mukundapur Transformer | 1012.07 | 357.41 | 307.28 |
| Mukundapur control Building | 1613.93 | 557.99 | 507.96 |
| Amuwa Transformer Site | 863.64 | 308.16 | 257.46 |
| Rangeli Transformer | 954.66 | 336.23 | 291.20 |
| Rangeli control Building | 1758.37 | 605.39 | 557.22 |
| Phatepur Transformer | 1354.95 | 470.28 | 423.71 |
| Phatepur Control building | 2358.61 | 805.14 | 757.80 |
| Biratchowk Transformer | 1661.60 | 573.97 | 523.71 |
| Biratchowk Control Building | 3569.31 | 1209.80 | 535.46 |
| Yedukuwa Control Building | 2711.94 | 925.21 | 872.14 |
| Yedukuwa Transformer | 3328.24 | 1131.01 | 1077.01 |
| Bhiman Control Building | 5258.44 | 1779.61 | 1712.61 |
| Bhiman Transformer | 4184.51 | 1421.10 | 1355.44 |
| Milanchowk Control Building | 3992.37 | 1357.62 | 1290.55 |
| Milanchowk Transformer | 3566.81 | 1214.58 | 1150.48 |
| Katari | 4298.05 | 1457.87 | 1394.90 |

c. Calculation of the bearing capacity by Hasen method (Mat Foundations)

| Location | Ultimate Bearing Capacity (kN/m ²) | Safe Bearing Capacity (kN/m ²) | Net Safe Bearing Capacity (kN/m ²) |
|-------------------------------|--|--|--|
| Phikal Transformer Site | 1179.21 | 410.48 | 366.95 |
| Phikal Control Building Site | 1569.59 | 542.00 | 495.00 |
| Aurahi Control Building | 886.82 | 314.46 | 267.33 |
| Aurahi Transformer | 1240.42 | 434.81 | 381.47 |
| Devdaha Control Building | 920.30 | 326.70 | 276.87 |
| Devdaha Transformer | 1070.66 | 377.29 | 326.29 |
| Simraungadha Transformer | 662.02 | 241.79 | 188.99 |
| Simraungadha Control Building | 2301.12 | 788.64 | 734.64 |
| Jitpur Transformer | 942.03 | 332.78 | 285.85 |
| Jitpur Control Building | 1326.47 | 460.82 | 414.16 |
| Mukundapur Transformer | 863.24 | 307.80 | 257.67 |
| Mukundapur control Building | 1380.31 | 480.12 | 430.08 |
| Amuwa Transformer Site | 761.73 | 274.19 | 223.49 |
| Rangeli Transformer | 740.32 | 264.79 | 219.75 |
| Rangeli control Building | 1515.02 | 524.27 | 476.11 |
| Phatepur Transformer | 1248.57 | 434.82 | 388.25 |
| Phatepur Control building | 2039.17 | 698.66 | 651.32 |
| Biratchowk Transformer | 1460.36 | 506.89 | 456.63 |
| Biratchowk Control Building | 3463.72 | 1174.60 | 482.41 |
| Yedukuwa Control Building | 2459.70 | 841.13 | 788.06 |
| Yedukuwa Transformer | 3005.37 | 1023.39 | 969.39 |
| Bhiman Control Building | 5521.17 | 1867.19 | 1800.19 |
| Bhiman Transformer | 4313.70 | 1464.17 | 1398.50 |
| Milanchowk Control Building | 4090.75 | 1390.41 | 1323.34 |
| Milanchowk Transformer | 3632.73 | 1236.55 | 1172.45 |
| Katari | 4457.80 | 1511.12 | 1448.15 |

Table 9: Results of the resistivity measurements

| Location | Resistivity ρ in Ω - m | Resistance R in Ω |
|-------------------------------|------------------------------------|--------------------------|
| Phikal Transformer Site | 20.00 | 0.40 |
| Phikal Control Building Site | 19.60 | 0.39 |
| Aurahi Control Building | 1.10 | 0.02 |
| Aurahi Transformer | 1.15 | 0.02 |
| Devdaha Control Building | 1.33 | 0.03 |
| Devdaha Transformer | 1.33 | 0.03 |
| Simraungadha Transformer | 1.50 | 0.03 |
| Simraungadha Control Building | 1.12 | 0.02 |
| Jitpur Transformer | 77.00 | 1.53 |
| Jitpur Control Building | 2.00 | 0.04 |
| Mukundapur Transformer | 2.00 | 0.04 |
| Mukundapur control Building | 2.30 | 0.05 |
| Amuwa Control Building | 3.90 | 0.08 |
| Amuwa Transformer Site | 3.70 | 0.07 |
| Rangeli Transformer | 2.10 | 0.04 |
| Rangeli control Building | 2.41 | 0.05 |
| Phatepur Transformer | 1.61 | 0.03 |
| Phatepur Control building | 1.29 | 0.03 |
| Biratchowk Transformer | 1.46 | 0.03 |
| Biratchowk Control Building | 1.61 | 0.03 |
| Yedukuwa Control Building | 1.24 | 0.02 |
| Yedukuwa Transformer | 1.19 | 0.02 |
| Bhiman Control Building | 32.00 | 0.64 |
| Bhiman Transformer | 19.10 | 0.38 |
| Milanchowk Control Building | 24.40 | 0.49 |
| Milanchowk Transformer | 29.70 | 0.59 |
| Katari | 40 | 0.8 |

and slight variation in the friction angle has greater effect on bearing capacity of the soil. Therefore, the factor of safety in the calculation of the bearing capacity is taken between 3 and 5. In a case; the excavated depth is left for a long time without filing the safe bearing capacity of the soil is determined by the formula

Safe bearing capacity of the soil = $q/F.S.$
Net safe bearing capacity of the soil = $q/F.S. - \gamma Z$
(Presented in Tables 7 and 8).

Resistivity measurements

Resistivity measurements were carried out only to explore the suitability of the sites for the proposed construction of substations in terms of earthing. As the substations will be equipped with transformer and switchyard it is necessary to have good conductive soil for the earthing purpose. Resistivity measurement showed that the sites are suitable for the construction of proposed substations. The resistance R of the site is calculated by using the resistivity (ρ) of the site measured by Earth Tester, MEGGER® DET 3/2.

The measured resistivity of the site is calculated by using the formula $\rho = 2\pi \times a \times R$.

Where; ρ = resistivity, a = distance between two electrodes; R = resistance. The resistivity data are for the depth up to 8m. The results of the resistance are presented in Table 9.

DISCUSSIONS

Site maps were prepared during the detail survey and in each map control building and switchyard location is fixed according to the orientation of the high voltage line. Boreholes were drilled using hand auger so if boulder were encountered, the boreholes were stopped at that depth. During the SPA test if the tip of the SPA was stopped by the boulder than SPA test was also stopped at that depth. The SPA test was once carried out in the nearby area to conform the presence of boulder and if the SPA is stopped once again than it was carried out up to that depth only. The bearing capacity obtained by the Hasen methods are recommended for the design as it is calculated by considering both cohesion and the friction angle of the soil. The length and breadth of the footings also control the bearing capacity of the soil so it is recommended that if the design is not similar as given above it

is recommended that bearing capacity should be recalculated after the detail design of the foundation. In the calculation of the bearing capacity, inclined loading and horizontal loading like earthquake shaking is not considered. The SPA test should be performed twice to prevent any biases from the sampling location. Once the standard value is obtained for large sample it can be used to determine the bearing capacity of the soil reducing the laboratory test.

CONCLUSIONS

The cohesion and friction angle are the most important and sensitive parameter in the calculation of the bearing capacity of the soil. Bearing capacity obtained by using 35 times N_c value and bearing capacity from laboratory data has a correlation coefficient of ~ 0.83 . So if the litholog of the area are available, N_c value obtained from the SPA test can be converted to ultimate bearing capacity. The resistivity data obtained in the field are suitable for the earthing as these are below the normal value. Resistivity measurement showed that the sites are suitable for the construction of proposed substations in terms of earthing.

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REFERENCES

- Bell, F.G., 1992, *Fundamental of Engineering Geology*. Aditya Books Private Ltd, New Delhi, 648p.
- Hansen, J. B., 1970, A revised and extended formula for bearing capacity. Danish Geotech. Inst. Bull., v., 28, pp. 5-11.
- Johnson, R. B. and DeGraff, J. V., 1988. *Principles of Engineering Geology*. John Wiley and Sons. Inc. Newyork, pp 76-177.
- Lambe, T.W., 1951, *Soil Testing for Engineers*. John Wiley & Sons, New York, 165p.
- Meyerhof, G. C., 1951, The ultimate bearing capacity of foundation. Geotechnique, v., 2, pp. 301-332.
- Operating instruction book 6171-524, The Megger® DET3/2 Digital Earth Tester
- Terzaghi, K. and Peck, R.B. 1967, *Soil Mechanics in Engineering Practice*. John Wiley & Sons, New York, 341p.